



Planning Permit Application

CITY OF BANDON PLANNING
 P.O. BOX 67
 555 HWY 101
 BANDON, OR 97411
 P:(541) 347-7922
 F:(541)347-1415

Permit Number: **22-112**

APPLICATION TYPE (select all that apply)		
<input type="checkbox"/> Annexation*	<input type="checkbox"/> Land Use Review*	<input type="checkbox"/> Subdivision*
<input type="checkbox"/> Certificate of Appropriateness (CoA)*	<input type="checkbox"/> Partition*	<input type="checkbox"/> Vacation*
<input type="checkbox"/> Comprehensive Plan or Zone Amendment*	<input type="checkbox"/> Plan Review (PR)	<input type="checkbox"/> Variance*
<input type="checkbox"/> Conditional Use Permit (CUP)*	<input type="checkbox"/> Planned Unit Development (PUD)*	<input type="checkbox"/> Zoning Compliance (ZC)
<input type="checkbox"/> Floodplain Development*	<input type="checkbox"/> Property Line Adjustment (PLA)*	<input type="checkbox"/> Other*
* Pre-application required		Total Fees: \$ 75.00

I. PROJECT LOCATION		
Street Address: 362 9th St (Wood Shop) (W) NO # 8th St SW Bandon OR 97411		
Map Number / Tax Lot(s): 1	Zone:	Floodplain: <input type="checkbox"/> Yes <input type="checkbox"/> No

II. APPLICANT'S INFORMATION (applicant is the primary party responsible for development)		
Applicant's Name: Chris Trevisiol	Phone: 541-347-4411	E-Mail: ctrevisiol@bandon.
Applicant's Mailing Address: 455 9th St SW Bandon OR 97411		

III. PROPERTY OWNER'S INFORMATION		
Property Owner's Name: Bandon School District	Phone: 541-347-4411	E-mail: SSchmerer@bandon.
Mailing Address: 455 9th St SW Bandon OR 97411		

IV. OTHER INFORMATION (APPLICANT'S REP, SURVEYOR, ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, ETC)		
Title:	Name:	
Email:	Phone:	
Title:	Name:	
Email:	Phone:	
Title:	Name:	
Email:	Phone:	

V. PROJECT DESCRIPTION		
Use: <input type="checkbox"/> Residential <input type="checkbox"/> Commercial <input checked="" type="checkbox"/> Other Public School		
*Please attach a short narrative that describes your proposed project and indicates the proposed use.		
FRAME IN and USE EXISTING play structure FOR CTE Wood Shop instruction AREA		

VI. SITE PLAN: Please see our "How to Create a Site Plan" and sample site plan document for requirements and tips on how to create your site plan. Plans must be drawn to scale and may be submitted electronically; printed copies must be submitted on 11x17, ledger size paper (larger or smaller paper sizes will not be accepted).


K12. OR. US

VII. PROPERTY OWNER SIGNATURE/AUTHORIZATION

- I have read the application and the attached documentation and I understand that my application may be delayed or deemed incomplete if I have provided insufficient information and documentation to allow for approval.
- I certify that the information provided in this application, including all submittals and attachments, is true and correct to the best of my knowledge.
- I understand and agree that all required inspections will be requested 2 business days in advance, and it is the applicant's responsibility to ensure required inspections have been requested, completed, and approved.
- I authorize the City of Bandon or its acting agent, to enter onto the subject property, as described in section "I. Project location".
- I authorize the following party(s) to act as applicant in regard to the attached application for the subject property described above.

X Applicant's Signature:  **Date:** 10-17-22

Property owner's signature required if applicant is not the property owner

X Property Owner's Signature:  **Date:** 10-17-22


Development Disclosure

The City of Bandon is obligated to report all ground disturbances within the City of Bandon to the Coquille Indian Tribe. Property owners and applicants must adhere to all conditions and requirements set out by the Coquille Indian Tribe, State Historic Preservation Office (SHPO) or both if required. Please be aware that state statutes and federal law govern how archaeological sites are to be managed. ORS 97.745 prohibits the willful removal, mutilation, defacing, injury, or destruction of any cairn, burial, human remains, funerary objects, or objects of cultural patrimony of a Native Indian. ORS 358.920 prohibits excavation, injury, destruction, or alteration of an archaeological site or object, or removal of an archaeological object from public or private lands.

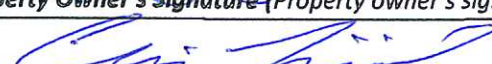
It is the property owner and applicant's responsibility to determine if additional permits from other agencies will be required, including but not limited to: Oregon State Building Codes, Oregon State Department of Environmental Quality, FEMA, Oregon State Fish and Wildlife and U.S. Fish and Wildlife. If additional permits are required, it is the responsibility of the property owner/applicant to obtain such permits and comply with their conditions of approval.

It is the property owner/applicant's responsibility to provide the City of Bandon all necessary legal documentation related to the property, including but not limited to: proof of ownership, receipts, deed restrictions, vacation records, easement records, etc.

I acknowledge, understand, and agree, that all relevant documentation will be provided to the City of Bandon, and that all required permits and consent will be obtained prior to the start of construction, with all conditions of approval adhered to.

X  **10-17-22**

Property Owner's Signature (Property owner's signature required if applicant is not the property owner) **Date**

X  **10-17-22**

Applicant's Signature **Date**

Staff's Signature of Intake:  Date: 10/25/22

Staff's Signature of Completeness:  Date: 10/25/22

Staff's Signature of Approval: _____ Date: _____



JOB: 42X50X16 POLE BUILDING
 SHEET NO.: _____ OF _____
 CALCULATED BY: RT DATE 8/17/22
 CHECKED BY: _____ DATE _____
 PROJECT NO.: BANDON ,OR

Client: BJS Metal & Lumber

Location: 455 9TH STREET SW BANDON OR 97411

Input: building dimensions and coefficients

Wind Direction = (Normal or Parallel to building ridge) >
 Wind Speed, V = **120** mph
 Bldg. Classification = **I**
 Exposure Category = **B**
 Ridge Height, hr = **23.25** feet
 Eave Height, he = **16.25** feet
 Building Width = **42** feet
 Building Length = **50** feet
 Roof Type = **Gable**
 Roof Pitch = **4**
 Enclosed? (Y/N) **yes**
 Bay Length: **10** feet
 Rafter Spacing: **2** feet
 Girt Spacing: **2** feet
 Embedment Depth: **4** feet
 Embedment Coef. **0.375**

Input: Loads

ROOF
 Dead Load: **5** psf
 Ground Snow Load (P_g) **25** psf
 Wall Dead Load = **2** psf
 Concrete Slab **yes**
 Overhang at eave: **2**
 Overhang at eave: **2**

Input: Material Characteristics

Snow Importance Factor I		
Risk Category	I	
I	0.8	
II	1	B
III	1.1	0.3
IV	1.2	0.8

MEMBER	Species	Size	b (in)	h (in)	F _v	E
RAFTER	DF#2	2x6	1.50	5.5	95	1600000
GIRT	DF#2	2x6	1.50	5.5	95	1600000
COLUMN	HF#2	6X8	6	8	140	1300000
COLUMN	HF#2	6X8	6	8	140	1300000

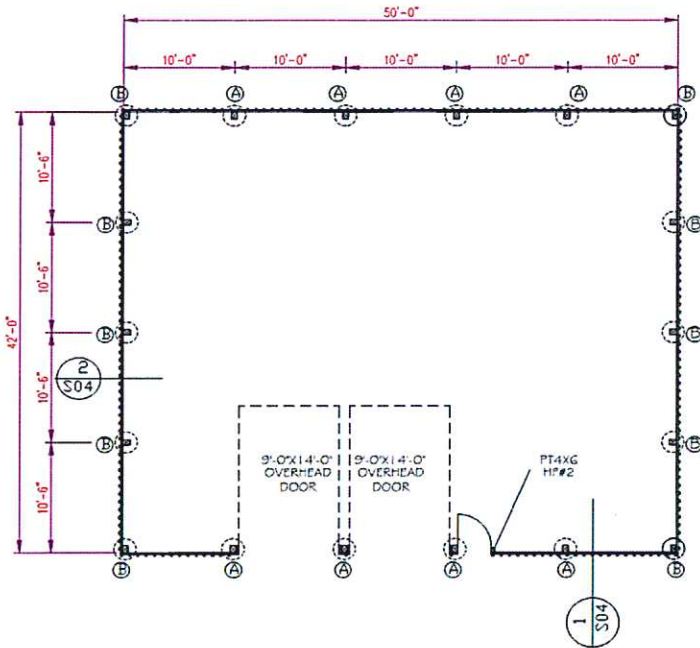
MEMBER	F _b	F _c	CF/F _b	CF/ F _c	C _d	C _r	C _f	C _{fu}
RAFTER	900.00	1300.00	1.30	1.00	1.15	1.15	1.15	-
GIRT	900.00	1300.00	1.30	1.00	1.60	1.15	1.10	1.15
COLUMN	575.00	570.00	1.00	1.00	1.60	1.00	1.00	-
COLUMN	575.00	570.00	1.00	1.00	1.60	1.00	1.00	-

Applicable Codes and Standards

- 2019 Oregon Structural Specialty Code (based on 2018 IBC)
- International Building Code (IBC2018 edition)
- ASCE-7-16
- 2015 NDS

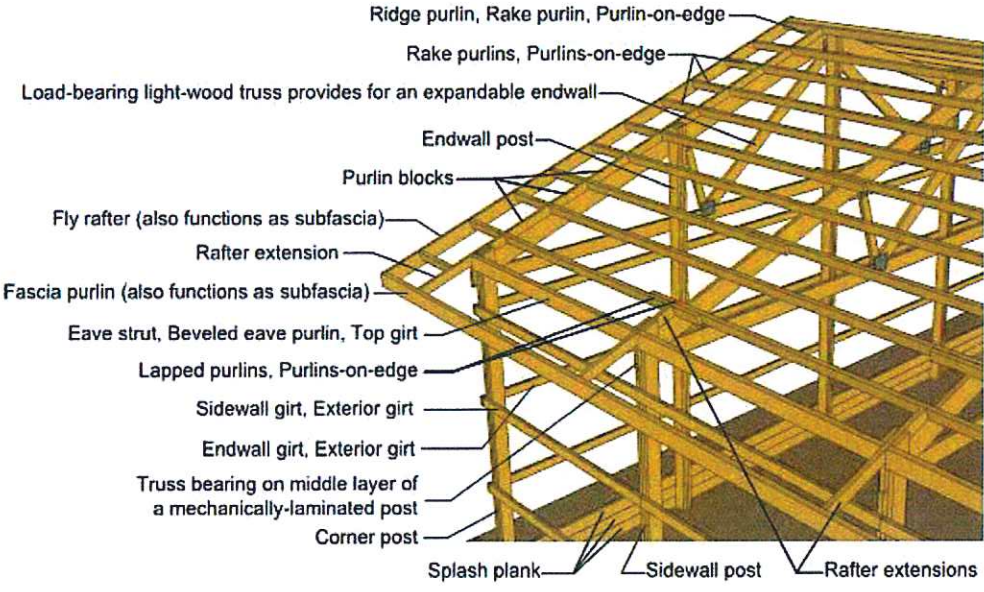


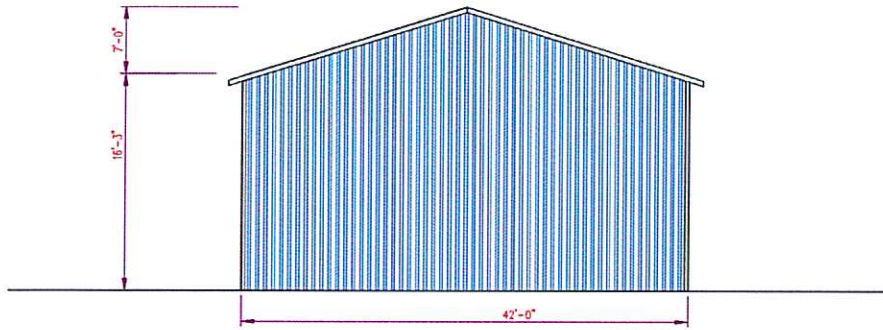
EXPIRES: 12/31/23



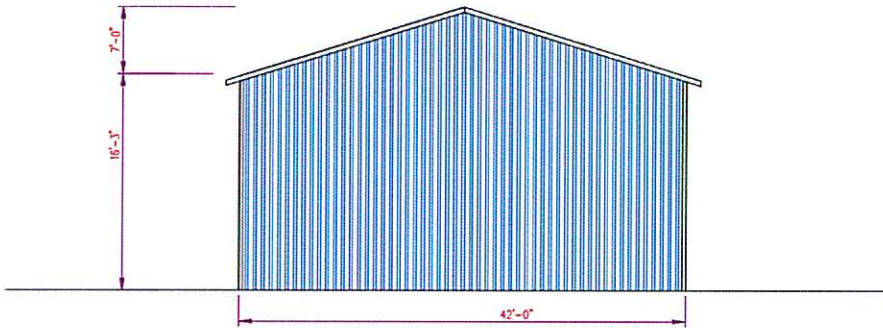
NOTE:
THIS BUILDING WAS PERMITTED AND BUILT AS ROOF ONLY WITH ALL FOUR SIDES OPEN.
THIS SCOPE OF THIS PERMIT/WORK IS TO ENCLOSE THE BUILDING AND PROVIDE 2 OVERHEAD DOORS AND ONE MAN DOOR

POST	POST SIZE	HOLE DIA (IN)	HOLE DEPTH (IN)	REMARKS
(A)	EXISTING POSTS P.T 6X6 HF#2	29	54	SEE DETAIL 2/S07
(B)	EXISTING POSTS P.T 6X8 HF#2	21	54	SEE DETAIL 2/S07

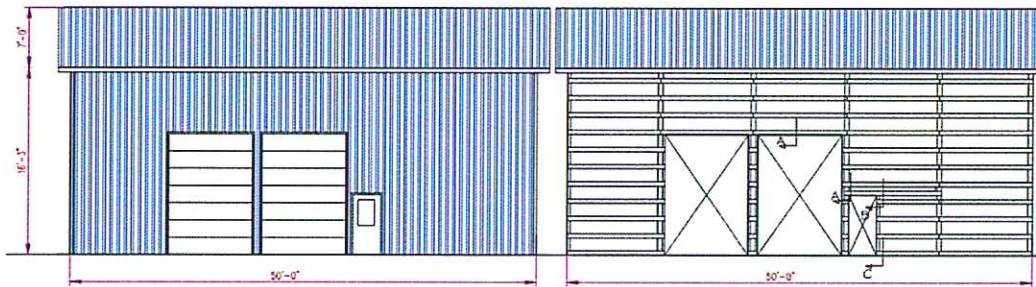




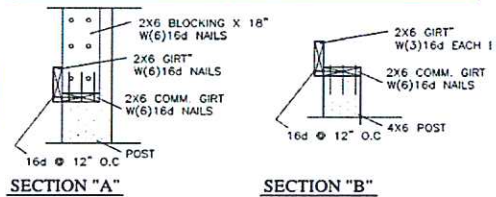
1
S08 LEFT ELEVATION



2
S09 RIGHT ELEVATION

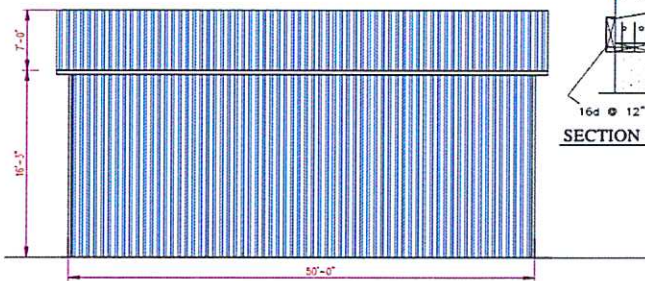


1
S07 FRONT ELEVATION

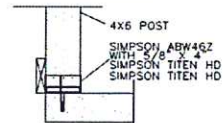


SECTION "A"

SECTION "B"



2
S07 BACK ELEVATION



SECTION "C"

snow design data

Thermal Factor; Ct

Thermal Condition	Ct
All structures	1
Structure kept just above freezing	1.1
Unheated Structures intentionally kept below freezing	1.2

Exposure Factor; Ce

Terrain Category	Terrain		
	Fully Exposed	Partially Exposed	Sheltered
B	0.9	1	1.2
C	0.9	1	1.1
D	0.8	0.9	1
Cs:	1		
Lu:	25	ft	
S:	4.00	ft	
γ:	17.25	pcf	
W	21	feet	

Flat Roof Snow Load

$p_f = 0.7 C_e C_t I p_g$	14.0	psf
Ground snow load; P_g	25	psf
Snow Important factor; I	0.8	
Snow exposure factor; C_e	1	
Thermal Factor; C_t	1	
Sloped Roof Snow Load (balanced)		
$L/B =$	1.19	
$p_s = C_s p_f$	14	psf
IF width "W" <= 20 feet.		
$p_{su} = I p_g$	20	psf
IF width "W" > 20		
$h_d = 0.43 \times (lu) / 3 \times (Pg + 10) / 4 - 15$	1.56	feet
$P_{suu} = h_d \times \gamma / (S)^{1/2}$	13.44	psf
$L_{un} = \{8 \times (S)^{1/2} \times h_d\} / 3$	8.31	feet
$P_{su} = p_{su} + p_s$	27.44	psf

MINIMUM DESIGN LOADS

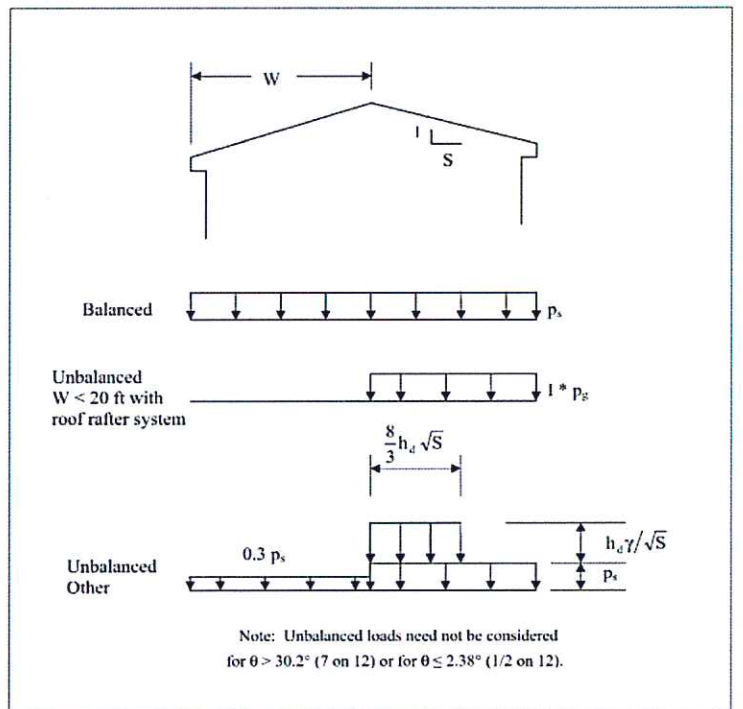


FIGURE 7-5 Balanced and Unbalanced Snow Loads for Hip and Gable Roofs.

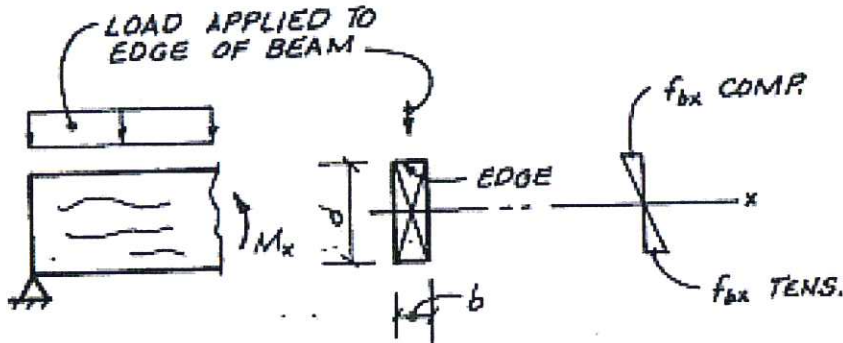
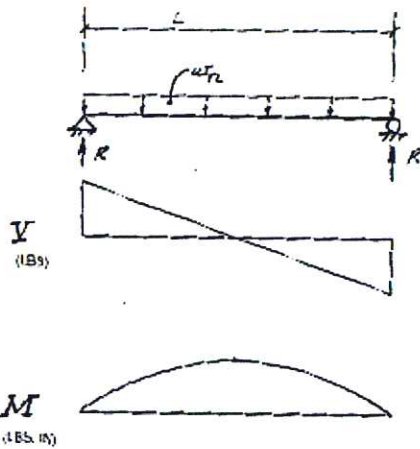
check rafters 2x6 rafter spacing = 2'-0"

Section Characteristics

Area:	8.25	in ²	S _{xx}	7.56	in ³
I _{xx}	20.80	in ⁴	S _{yy}	2.06	in ³
I _{yy}	1.55	in ⁴	F _b	1547.3	psi

Roof Slope	18.44	Degree
Roof Rafter Span	9.25	feet
Roof Height:	7.00	feet
Building Height:	19.75	feet
Minimum roof snow load		
Design snow load	20.00	psf

Applied Load (plf)	Bending Moment (lbs.in)		Shear Force (lbs)		Bending Stress(psi)		Shear Stress(psi)	
$w = (ws + wdl) \times s$	$M = w L^2/8$		$V = w L / 2$		$f_b = M/S$		$v = (2/3)V/A$	
49.90	6404.4		230.79		846.80		41.96	
Deflection (in)					PASS		PASS	
$\Delta = (5/384)((wL^4 \times 1728)/EI)$			0.25					
Allowable Deflection (in)			0.62		PASS			





plan review consultant ■ seismic brace ■ structural design ■ structural evaluation

JOB: 42X50X16 POLE BUILDING
 SHEET NO.: _____ OF _____
 CALCULATED BY: RT DATE 8/17/22
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wind loads

Height above ground level, z		Exposure		
ft	(m)	B	C	D
0-15	(0-4.6)	0.70	0.85	1.03
20	(6.1)	0.70	0.90	1.08
25	(7.6)	0.70	0.94	1.12
30	(9.1)	0.70	0.98	1.16
40	(12.2)	0.76	1.04	1.22
50	(15.2)	0.81	1.09	1.27
60	(18)	0.85	1.13	1.31

Roof Angle θ (degrees)	LOAD CASE A											
	Building Surface											
	1	2	3	4	1E	2E	3E	4E				
0-5	0.40	-0.69	-0.37	-0.29	0.61	-1.07	-0.53	-0.43				
20	0.53	-0.69	-0.48	-0.43	0.80	-1.07	-0.69	-0.64				
30-45	0.56	0.21	-0.43	-0.37	0.69	0.27	-0.53	-0.48				
90	0.56	0.56	-0.37	-0.37	0.69	0.69	-0.48	-0.48				

Risk Category	Importance Factor
I	0.87
II	1
III	1.15
IV	1.15

Notes:

- The velocity pressure exposure coefficient K_z may be determined from the following formula:
 For $15 \text{ ft} \leq z \leq z_g$ For $z < 15 \text{ ft}$
 $K_z = 2.01 (z/z_g)^{2.6}$ $K_z = 2.01 (15/z_g)^{2.6}$
 Note: z shall not be taken less than 30 feet in exposure B.
- α and z_g are tabulated in Table 26.9-1.
- Linear interpolation for intermediate values of height z is acceptable.
- Exposure categories are defined in Section 26.7.

$V_{wind \text{ ult}}$ **120.0 mph**
 k_z **0.700**
 K_{zt} **1**
 kd **0.85**
 $q_{h \text{ ASD}} = 0.00256 k_z k_{zt} k_d V_{wind}^2 \times 0.6$ **13.16 psf (ASD)**

$P = q_h \{9GC_{pf}\} - (Gc_{pi})$

$q_h \text{ (wall) min, ASD} = 16 \text{ psf} \times 0.6 = 9.6 \text{ psf} = 230.4 \text{ lbs}$
 $q_h \text{ (roof) min, ASD} = 8 \text{ psf} \times 0.6 = 4.8 \text{ psf} = 201.60 \text{ lbs}$

Girt Trib. Area = 24 SF
 Roof Trib Area = 42 SF
 lbs NOT Govern
 lbs NOT Govern

Eave Wall

Windward		leeward	
$GC_{p_{fww}}$	0.53	$GC_{p_{flw}}$	-0.43
$q_{ww} = q_h GC_{p_{fww}}$	6.98 psf	$q_{lw} = q_h GC_{p_{flw}}$	-5.66 psf
$GC_{p_{fww}} \text{ (corner)}$	0.80	$GC_{p_{flw}} \text{ (corner)}$	-0.64
$q_{ww} = q_h GC_{p_{fww}}$	10.53 psf	$q_{lw} = q_h GC_{p_{flw}}$	-8.42 psf

Roof

Windward		leeward	
$GC_{p_{fwr}}$	-0.69	$GC_{p_{flr}}$	-0.48
$q_{ww} = q_h GC_{p_{fwr}}$	-9.08 psf	$q_{lw} = q_h GC_{p_{flw}}$	-6.32 psf
$GC_{p_{fwr}} \text{ (corner)}$	-1.07	$GC_{p_{flr}} \text{ (corner)}$	-0.69
$q_{ww} = q_h GC_{p_{fwr}} \text{ (corner)}$	-14.08 psf	$q_{lw} = q_h GC_{p_{flw}} \text{ (corner)}$	-9.08 psf

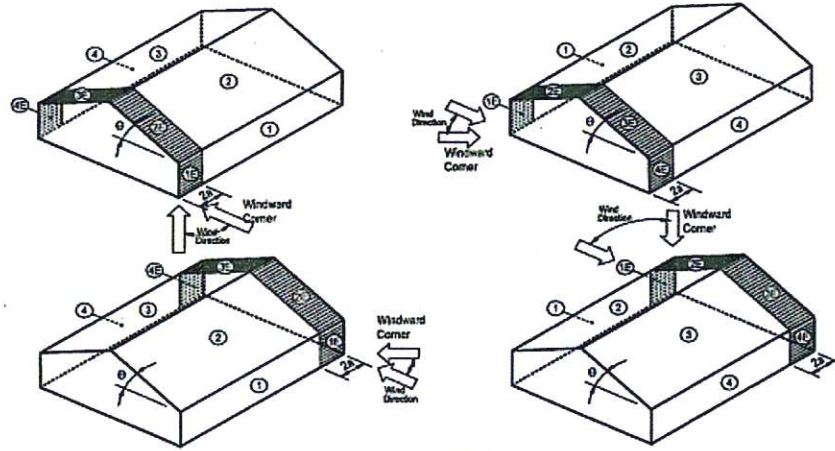
Roof Angle θ (degrees)	LOAD CASE B											
	Building Surface											
	1	2	3	4	5	6	1E	2E	3E	4E	5E	6E
0-90	-0.45	-0.69	-0.37	-0.45	0.40	-0.29	-0.48	-1.07	-0.53	-0.48	0.61	-0.43

Gable Wall

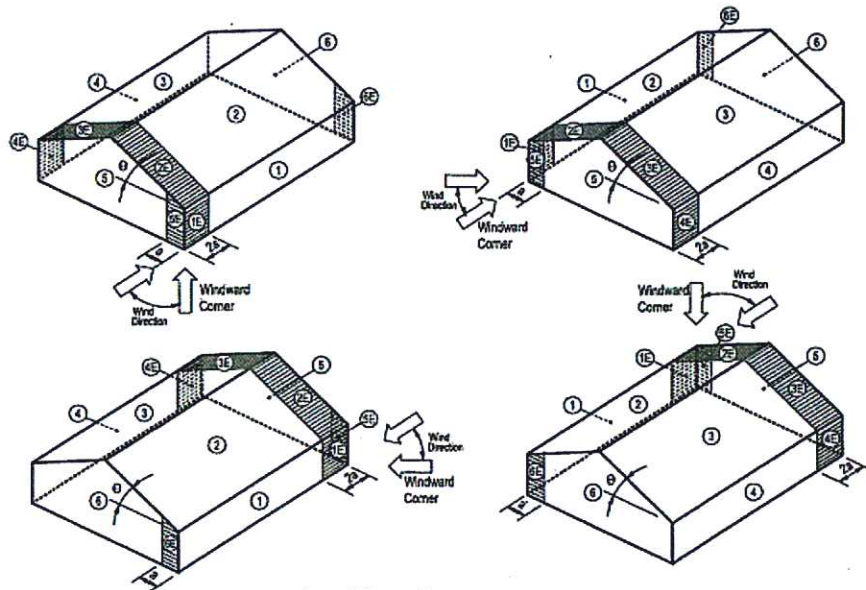
Windward		leeward	
$GC_{p_{fwwg}}$	0.40	$GC_{p_{flwg}}$	-0.29
$q_{ww} = q_h GC_{p_{fwwg}}$	5.26 psf	$q_{lw} = q_h GC_{p_{flw}}$	-3.82 psf

Wall elements		Roof elements	
$GC_{p_{fw}}$	0.61	$GC_{p_{fr}}$	0.00
$q_{ww} = q_h GC_{p_{fw}}$	8.03 psf	$q_{lw} = q_h GC_{p_{fr}}$	0.00 psf

Internal Pressure		Gc_{pi}	
$q_{ww} = q_h GC_{pi}$	-2.37 psf	-0.18	0.18



Load Case A



Load Case B

Basic Load Cases

Wall girts

2x6 girt

Section Characteristics

Area:	8.25	in ²
I _{xx}	20.80	in ⁴
I _{yy}	1.55	in ⁴
Girt Span	9.50	ft

S _{xx}	7.56	in ³
S _{yy}	2.06	in ³
F _{bxx}	2368.1	psi
Girt eff. Area =	24	SF

USE STRONGBACK: **NO**

F_{byy} = 2723.3 psi
w (lbs) = 309.5 lbs

Applied Load (plf)	Bending Moment (lbs.in)	Shear Force (lbs)	Bending Stress(psi)	Shear Stress(psi)
$w = (ww+GCpi) \times s$	$M = w L^2 / 8$	$V = w L / 2$	$f_b = M/S$	$v = (2/3)V/A$
25.79	3491.9	122.52	1692.65	22.28
Deflection (in)			PASS	PASS
$\Delta = (5/384)(wL^4 \times 1728)/EI$		0.714		
Allowable Deflection (in)		1.267	PASS	

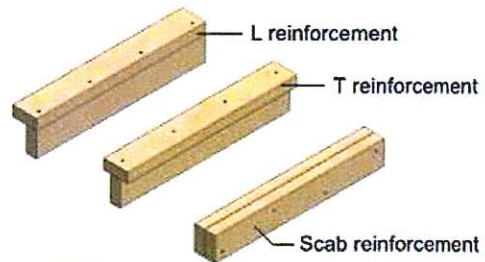
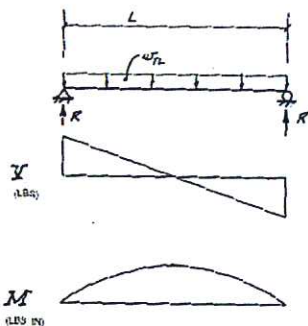
2x4 Strong back for the flat girt

AREA #	b(in)	d(in)	bxd (in ²)	yi (in)	Aix yi	yy (in)	lyy (in ⁴)	Aixyyi ² (in ⁴)	Es: (ksi)	Ew: (ksi)
2X6	5.5	1.5	8.2500	0.75	6.19	0.97	1.55	7.80	1600	1600
2X4	1.5	3.50	5.2500	3.25	17.06	-1.53	5.36	12.25		
Sum			13.50	Sum	23.25		6.91	20.05		

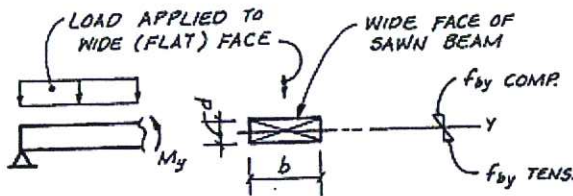
y _t	1.72	in	n	1			S _b	8.22	in ³
y _b	3.28	in	l	26.96	in ⁴		S _t	15.65	in ³

AREA #	b(in)	d(in)	bxd (in ²)	yi (in)	Aix yi	yy (in)	lyy (in ⁴)	Aixyyi ² (in ⁴)	Es: (ksi)	Ew: (ksi)
2X6	5.5	1.5	8.2500	0.75	6.19	0.33	1.55	0.89	29000	1600
29 gauge	24	0.0142	6.3031	1.51	9.50	0.43	0.00	1.70		
Sum			14.55	Sum	15.69		1.55	2.59		

y _t	1.0779	n	18.13
y _b	0.44		



Types of individual member reinforcement.



diaphragm analysis

EMBEDMENT LENGTH, d	54	IN
WALL HEIGHT, H1	195	IN
AREA	48	IN ²
SECTION MODULUS	64.00	in ³
INERTIA	256.00	IN ⁴
P = 0.4 Pult =	2000	lbs
P-1	170	plf
Ds =	0.35	in
b =	12	feet
b' =	10.00	feet
a =	10	feet
mD =	0.8	
DT =	5	psf
DB =	0	psf

$$k = 6EI / (H^2(0.7d+H)) \quad 225.6 \text{ lbs/in}$$

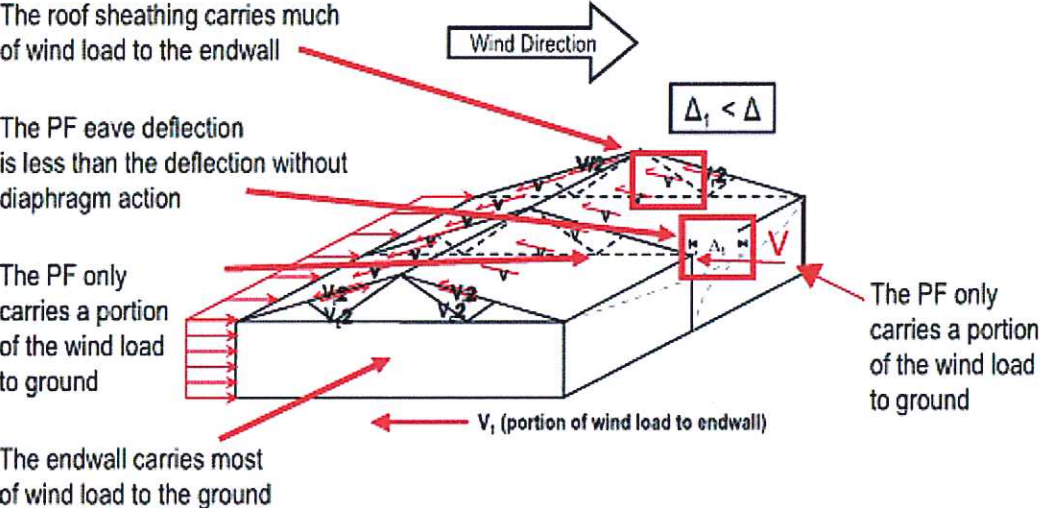
$$C1 = 0.5 \times (P/D) \times (a/b) = \quad 2381.0 \text{ lbs/in}$$

$$c_n = 2C_1(b'/b)(a/s_r) \times \cos^2 \alpha \quad 3571.43 \text{ lbs/in}$$

$$k/c_n \quad 0.0632$$

Number of frames: **6**

POST FRAME (PF) RESPONSE TO LATERAL LOAD WITH DIAPHRAGM ACTION





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Restraining Force

$$R = sf[(h1(q_{ww} - q_{lw})(2.8d + 3H1)/8(0.7d + H1)) + H2(q_{wr} - q_{lr})]$$

Shear at top of column

$$V = (1/2)[RmD + sf\{(H1(q_{ww} + q_{lw})(2.8d + 3h1)/(8(0.7d + h1)) - H2(q_{ww} - q_{lw})\}]$$

Moment at the groundline

$$M_1 = k_1 H_1 (V - (s_r q_{ww} H1/2))$$

$$M_2 = k_2 V^2 / 2sfq_{ww}$$

$$P_r = (s_r W/2)[(s/2) + D_r + D_B + (3q_{wr} + q_{lr})/4] + (H2/2W)[R.mD + s_r H_2 (q_{lr} - q_{ww})]$$

1691.99 lbs.

276.67 lbs

k1 = 0.9

k2 = 0.9

-44639 lbs.in

5926.00 lbs.in

reaction

5738.50 lbs

Wind pressure at eave wall (min. = 16 psf x 0.6 = 9.6 psf)

wind pressure at eave height

Win. Roof + Lee. Roof

W1= 12.6 psf

R1= 1.0 psf

Total= 13.6 psf

Shear Diaphragm

$$I = [(K(q_{ww} - q_{lw})H_{1L}) + (q_{wr} - q_{lr})H_{2L}]/2W$$

77.1 lbs/ft

$$V = I \times \text{building width} = 3237 \text{ lbs}$$

Transverse Direction

Double Sheathing walls No

#9 screws at 9" O.C shear (lbs)= 176.0

OK

based on %50 of #9 screw to 14 gage side member per (NDS 2015 Table 12M)

	doors/ window	length of wall (feet)	shear (plf)	
Wall	0	42	77	USE 29 metal gauge sheathing
Wall	0	42	77	USE 29 metal gauge sheathing

See Pier Design

Longitudinal Direction

Double Sheathing walls Yes

v = 12.6 psf

AREA: 581.25 ft²

v = 3671.8 lbs

Available Wall Length; L1 =

50 feet

Available Wall Length; L2 =

29 feet

vw1= 73.435 plf

USE 29 metal gauge sheathing

vw2= 126.61 plf

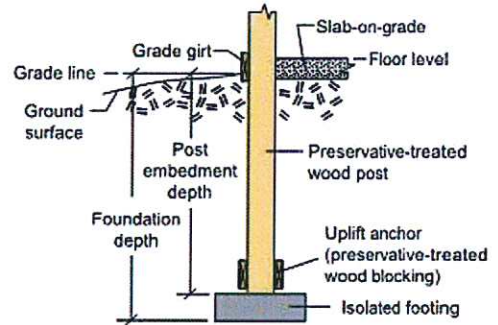
USE 29 metal gauge sheathing

Foundation

LOFT **no**

NO LOFT

REACTION : 5738.50 lbs
 Fbearing = 1950.00 plf
 Arequired Area = 2.94 ft²
 Hole Diameter 23.23 in
 Area Used 3.14 ft²
 Soil Stress 1827.55 psf
 use **24** in



Post foundation featuring a preservative-treated wood blocks for uplift anchorage.

Post Design

Compression $F_c^* = C_d C_r C_F F_c$ 912.00 psi
 Bending $F_b^* = C_d C_r C_F C_d F_c$ 920.00 psi
 Slenderness Ratio L_c/d 17.1000 **PASS**

	(psi)	(psi)	$(F_c/F_c^*)^2 + F_b/F_b^*$
STRESS	119.55	697.49	
ALLOWABLE STRESS	912.00	920.00	0.78 PASS

$F_{cE} = (K_{cE} E') / (L/d)^2$

Kc = **0.8**

1333.74 psi

F_{cE}/F_c^*

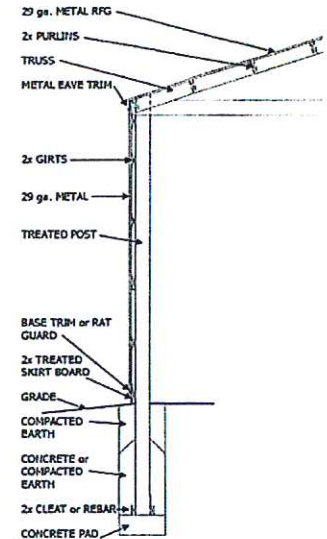
1.46

$C_p = \{ [1 + (F_{cE}/F_c^*)] / 2c \} - \{ [(1 + (F_{cE}/F_c^*) / 2c)^2 - (F_{cE}/F_c^*)] / c \}^{1/2}$

0.80

$F_c = C_p F_c^*$

733.07 psi



	(psi)	(psi)	$(F_c/F_c^*)^2 + F_b/(F_b^*(1-f_j/F_{cE}))$
	119.55	697.49	
ALLOWABLE STRESS	733.07	920.00	0.9 PASS
ACTUAL STRESS	119.55	46.30	
ALLOWABLE STRESS	733.07	920.00	0.1 PASS

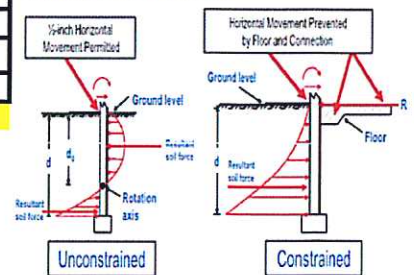
Post buried depth

Assumed Depth h **54.00** in
 Lateral Pressure p 150.00 psf
 DIAMETER: b **24.00** in
 S= 1.33 x 2 x p 399.00 psf

post depth in the ground	
non-constrained	constrained
$d^2 = [3.51 V_g / (Sb)] [1 + (1 + (0.62 M_g Sbd) / V_g^2)^{1/2}]$	$d^3 = 4.25 M_g / Sd$
44.79	24.78

EMBEDMENT LENGTH IS EQUAL TO: **44.79** inches **Use 54"**

Two primary post/pier embedment types:



common truss connections

Number of Bolts N = **3** **3/4 DIAMETER BOLT**
Reaction 2869.25 lbs

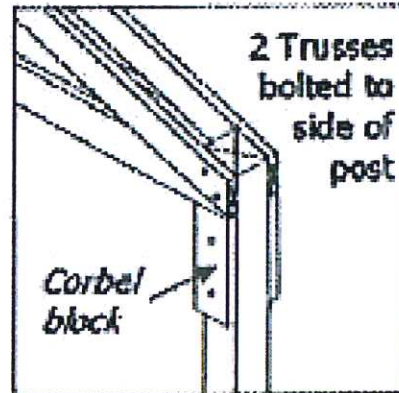
DIA. (in)	F _{yb} (psi)	t _s (in)	F _{all} (psi)	ALPH	F _{es} (psi)	C _D	t _m (in)	F _{em} (psi)
0.75	45000	1.5	4800	0	2250	1.15	6	2250
R _e	1+R _e	1+2R _e	2+R _e	R _t =t _m /t _s	1+R _t	1+R _t +R _t ²	K _a	
1	2	3	3	4	5	21	1	

k1	1.3406	lbs
k2	1.151	lbs
k3	2.742	lbs
I (Z) Im	2531.3	lbs
I (Z) Is	632.81	lbs
II (Z) Is	942.6	lbs
II (Z) Im	1213.5	lbs
III (Z) Is	722.9	lbs
III (Z) Im	1021.2	lbs
Z	632.8	lbs
Add. Nails	6	957.6000

Z' = ZC_DC_MC_g 2183.2 lbs
Capacity of a bolt: 728 lbs

PASS

COMMON TRUSS CONNECTIONS



gable wall post check

Post Design	use: HF	6X6		
Compression	$F_c^* = C_d C_r C_F F_c$		912.00	psi
Bending	$F_b^* = C_d C_r C_F C_{df} F_c$		920.00	psi
Slenderness Ratio	Lc/d		26.00	PASS
Post tributary width			12	feet
Post unbraced height			16.25	feet
Dead Load			297	lb
Wind Load			1200	lb
Snow Load			96.33	plf
Moment			25438.32	lb.inch
$V_g =$			578.0068762	lb.inch
Post width			6	in
Post height			6	in
AT GROUNDLINE			$(F_c/F_c^*)^2 + F_b/F_b^*$	
	(psi)	(psi)		
ACTUAL STRESS	41.58	706.62		
ALLOWABLE STRESS	912.00	920.00	0.77	PASS

$K_{ce} = 0.3$
 $K_c = 0.8$
 $F_{ce} = (K_{ce} E') / (L/d)^2 = 1333.74$
 $F_{ce} / F_c^* = 1.46$
 $C_p = \{ [1 + (F_{ce} / F_c^*) / 2c] - [\{ (1 + (F_{ce} / F_c^*) / 2c)^2 - (F_{ce} / F_c^*) / c \}]^{1/2} \} = 0.80$
 $F_c = C_p F_c^* = 733.07$

ABOVE GROUNDLINE			
	(psi)	(psi)	RATIO OF STRESS
ACTUAL STRESS	41.58	706.62	
ALLOWABLE STRESS	733.07	920.00	0.8
ACTUAL STRESS	41.58	353.31	
ALLOWABLE STRESS	733.07	920.00	0.4

gable post embedment check

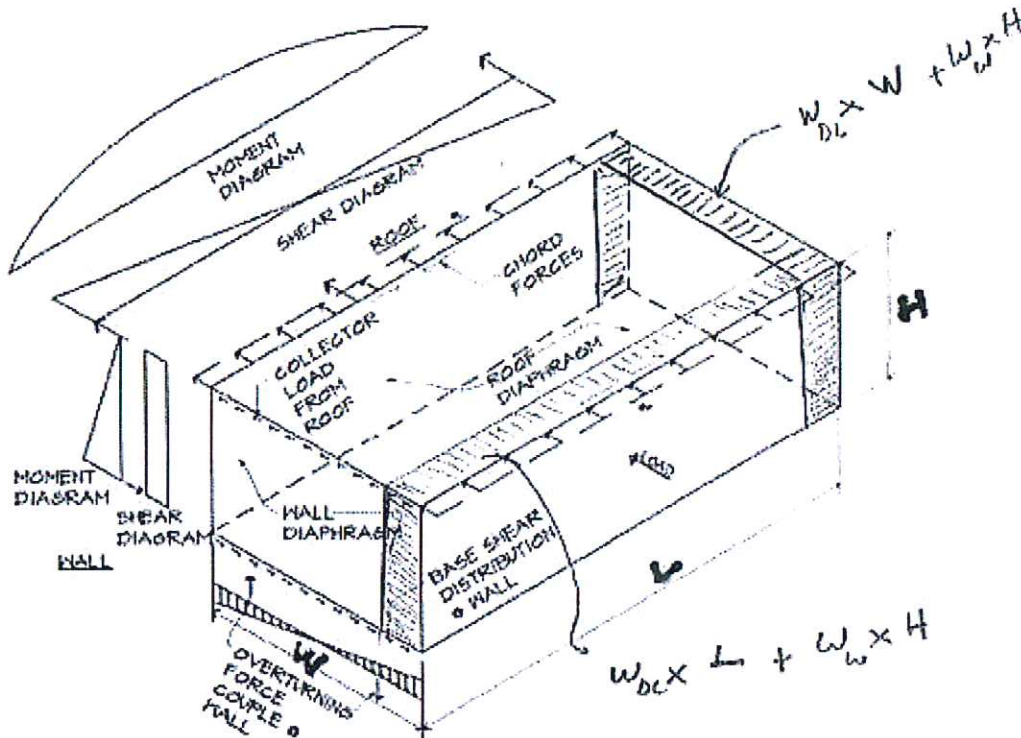
Assumed Depth, h = 54 in
 Lateral Pressure, p = 150 psf
 Diameter, b = 21 in
 $S = 1.33 \times 2 \times p = 399$ psf

post depth in the ground	
non-constrained	constrained
$d^2 = [3.51 V_g / (Sb)] [1 + (1 + (0.62 M_g Sbd) / V_g^2)]^{1/2}$	$d^2 = 4.25 M_g / Sd$
44.13	20.54
EMBEDMENT LENGTH IS EQUAL TO = 44.13 in Use 54"	

EARTHQUAKE /SEISMIC ANALYSIS

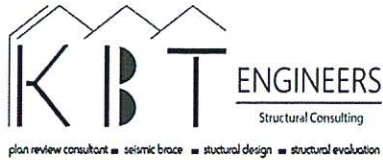
Risk Category:	I	
Site Class:	D	
$S_{ds} = (2/3)S_{ms} =$	1.62	
$S_{d1} = (2/3)S_{m1} =$	0.43	
S_{d1}	D	$S_{d1} > 0.2$

R	4		
$C_s (LRFD) = S_{ds}/R$	0.4		
$C_s (ASD) = S_{ds}/R \times 0.7$	0.3		
$V_{(ASD)} = C_s W$	68.3	plf	Seismic Govern OK LESS THAN 176 PLF
$w_{diff} \times width + 2H_{wall} w_{dlw/2}$	240	lbs	
$w_{diff} \times width + 2H_{wall} w_{dlw/2}$	391.1	lbs	
$V_{(ASD)} = C_s W$	88.8	plf	Seismic Govern OK LESS THAN 176 PLF
$w_{diff} \times length + 2H_{wall} w_{dlw/2}$	312.5	lbs	



Diaphragm Factor mD

k/ch	Number of frames									
	3	4	5	6	7	8	9	10	11	
0.0001	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
0.0002	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
0.0003	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
0.0004	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
0.0005	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99
0.0006	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99
0.0007	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99
0.0008	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99
0.0009	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99	0.99	0.99
0.0010	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99	0.99	0.99
0.0020	1.00	1.00	1.00	0.99	0.99	0.99	0.98	0.98	0.98	0.98
0.0030	1.00	1.00	0.99	0.99	0.99	0.98	0.98	0.97	0.96	0.96
0.0040	1.00	1.00	0.99	0.99	0.98	0.98	0.97	0.96	0.95	0.95
0.0050	1.00	1.00	0.99	0.99	0.98	0.97	0.96	0.95	0.94	0.94
0.0060	1.00	0.99	0.99	0.98	0.97	0.97	0.95	0.94	0.93	0.93
0.0070	1.00	0.99	0.99	0.98	0.97	0.96	0.95	0.93	0.92	0.92
0.0080	1.00	0.99	0.98	0.98	0.97	0.95	0.94	0.93	0.91	0.91
0.0090	1.00	0.99	0.98	0.97	0.96	0.95	0.93	0.92	0.90	0.90
0.0100	1.00	0.99	0.98	0.97	0.96	0.94	0.93	0.91	0.89	0.89
0.0110	0.99	0.99	0.98	0.97	0.95	0.94	0.92	0.90	0.88	0.88
0.0120	0.99	0.99	0.98	0.97	0.95	0.93	0.91	0.89	0.87	0.87
0.0130	0.99	0.99	0.97	0.96	0.94	0.93	0.90	0.88	0.86	0.86
0.0140	0.99	0.99	0.97	0.96	0.94	0.92	0.90	0.87	0.85	0.85
0.0150	0.99	0.99	0.97	0.96	0.94	0.92	0.89	0.87	0.84	0.84
0.0160	0.99	0.98	0.97	0.95	0.93	0.91	0.88	0.86	0.83	0.83
0.0170	0.99	0.98	0.97	0.95	0.93	0.91	0.88	0.85	0.82	0.82
0.0180	0.99	0.98	0.97	0.95	0.92	0.90	0.87	0.84	0.81	0.81
0.0190	0.99	0.98	0.96	0.95	0.92	0.90	0.87	0.84	0.80	0.80
0.0200	0.99	0.98	0.96	0.94	0.92	0.89	0.86	0.83	0.79	0.79
0.0210	0.99	0.98	0.96	0.94	0.91	0.89	0.85	0.82	0.79	0.79
0.0220	0.99	0.98	0.96	0.94	0.91	0.88	0.85	0.81	0.78	0.78
0.0230	0.99	0.98	0.96	0.93	0.90	0.88	0.84	0.81	0.77	0.77
0.0240	0.99	0.98	0.95	0.93	0.90	0.87	0.83	0.80	0.76	0.76
0.0250	0.99	0.98	0.95	0.93	0.90	0.87	0.83	0.79	0.75	0.75
0.0260	0.99	0.97	0.95	0.93	0.89	0.86	0.82	0.79	0.75	0.75
0.0270	0.99	0.97	0.95	0.92	0.89	0.86	0.82	0.78	0.74	0.74
0.0280	0.99	0.97	0.95	0.92	0.89	0.85	0.81	0.77	0.73	0.73
0.0290	0.99	0.97	0.94	0.92	0.88	0.85	0.81	0.77	0.72	0.72
0.0300	0.99	0.97	0.94	0.92	0.88	0.84	0.8	0.76	0.72	0.72
0.0310	0.98	0.97	0.94	0.91	0.88	0.84	0.8	0.75	0.71	0.71
0.0320	0.98	0.97	0.94	0.91	0.87	0.84	0.79	0.75	0.70	0.70
0.0340	0.98	0.97	0.94	0.91	0.86	0.83	0.78	0.74	0.69	0.69
0.0360	0.98	0.97	0.93	0.90	0.86	0.82	0.77	0.72	0.67	0.67
0.0380	0.98	0.96	0.93	0.90	0.85	0.81	0.76	0.71	0.66	0.66
0.0400	0.98	0.96	0.93	0.89	0.84	0.80	0.75	0.70	0.65	0.65
0.0420	0.98	0.96	0.92	0.89	0.84	0.79	0.74	0.69	0.64	0.64
0.0440	0.98	0.96	0.92	0.88	0.83	0.79	0.73	0.68	0.63	0.63
0.0460	0.98	0.96	0.91	0.88	0.82	0.78	0.72	0.67	0.61	0.61
0.0480	0.98	0.95	0.91	0.87	0.82	0.77	0.71	0.66	0.60	0.60
0.0500	0.98	0.95	0.91	0.87	0.81	0.76	0.70	0.65	0.59	0.59
0.0600	0.97	0.94	0.89	0.84	0.78	0.73	0.67	0.60	0.54	0.54
0.0700	0.97	0.93	0.88	0.82	0.75	0.69	0.62	0.56	0.5	0.5
0.0800	0.96	0.93	0.86	0.80	0.72	0.66	0.59	0.53	0.46	0.46
0.1000	0.95	0.91	0.83	0.76	0.68	0.61	0.53	0.46	0.40	0.40



JOB: 42X50X16 POLE BUILDING
 SHEET NO.: _____ OF _____
 CALCULATED RT DATE 8/17/22
 CHECKED BY: _____ DATE _____
 PROJECT NO. BANDON ,OR

Use Alu- Tuff Roof Diaphragm Panel Type

Panel	Capacity (plf)	Ultimate(lb)	0.4 Pu (lb)
P-1	170	5000	2000
P-2	170	5250	2100
P-3	240	9000	3600

P-1: Purlins recessed and installed with Kant-Sag JDS-24 purlin hangers. Two 8d common nails intalled in top of purlin hunger. Four 8d galvanized nails inatalled in each purlins end. Four additional 8d nails intalled in the wide faces of rafter, two each side.
 Fasteners:

Field: Typical industry fastener pattern with #12x1.5 screw head with neoprene washer

P-2: Purlins on-edge on top of truss rafters and nailed with one six-inch, 0.177 in diameter (60d) threaded and hardened nail. Purlins lapped one foot each side of the trussrafter providing a two foot lap at joining purlins.

Fasteners:

Field Typical industry fastener pattern with #12x1.5 screw head with neoprene washer

P-3: Purlins recessed and installed with Kant-Sag JDS-24 purlin hangers. Two 8d common nails in top of purlin hunger. Four 8d galvanized nails inatalled in each purlins end. Four additional 8d nails intalled in the wide faces of rafter, two each side.

Fasteners:

Field Typical industry fastener pattern with #12x1.5 screw head with neoprene washer

Stitcl # 12 x 5/8 in. self-tapping screw, hex head with neoprene washer, 12 in. on center

Gable End Attach steel to gable end rafters with field fasteners 12 in. on-center

Powerpanel End-Wall Diaphragm Constructions

WALL TYP.	ALLOW. SHEAR (LBS/FT)	
	12FT. HIGH	20 FT HIGH
Q1	170.0	120
Q2	280.0	270
Q3	315.0	280

The end-wall diaphragms tested used the 26 or 29 gauge Powerpanel profile manufactured by alumax building products. In addition to the specific requirement for each construction Q1 through Q5, the end-wall diaphragm systems tested have special requirements as follows.

1. Splash board is 2 x 8 or larger
2. The splash board is attached to each end-wall post with (7)20d
3. The end-wall diaphragm is attached directly to the truss top chord. Full length steel sheets are required.
4. The diaphragm construction Q1 through Q5 are fastened to the splash board and truss top chord with # 12 x 1-5/8" screws on both sides of major ribs. At all intermediate girts the same screws are required on one side of the major ribs.
5. Girts (2x4 or lager) are atatched flatwise with (2) 20d nails at each post location
6. All steel is 29 gauge Powerpanel except for Construction Q-4 that uses 26 gauge Powerpanel.

Q1: In addition to requirements 1 through 6, the girt spacing is 36" or less o.c

Q2: In addition to requirements 1 through 6, #12 x 1/2" stitich screws, hex head with neoprene washer, 8 in. o.c, are required. The girt spacing is 36" or less o.c

Q3: In addition to requirements 1 through 6, #12 x 1/2" stitich screws, hex head with neoprene washer, 8 in. o.c, are required. Install 2x4 blocking on the end-wall posts where the end-wall metal cladding ends (at sidewall and door faces) and fasten steel to blocking with # 12 x 1-5/8" screws 8" o.c. Install blocking with 20d nail, each end, not less than 2 in. from block ends. The girt spacing is 36" or less o.c

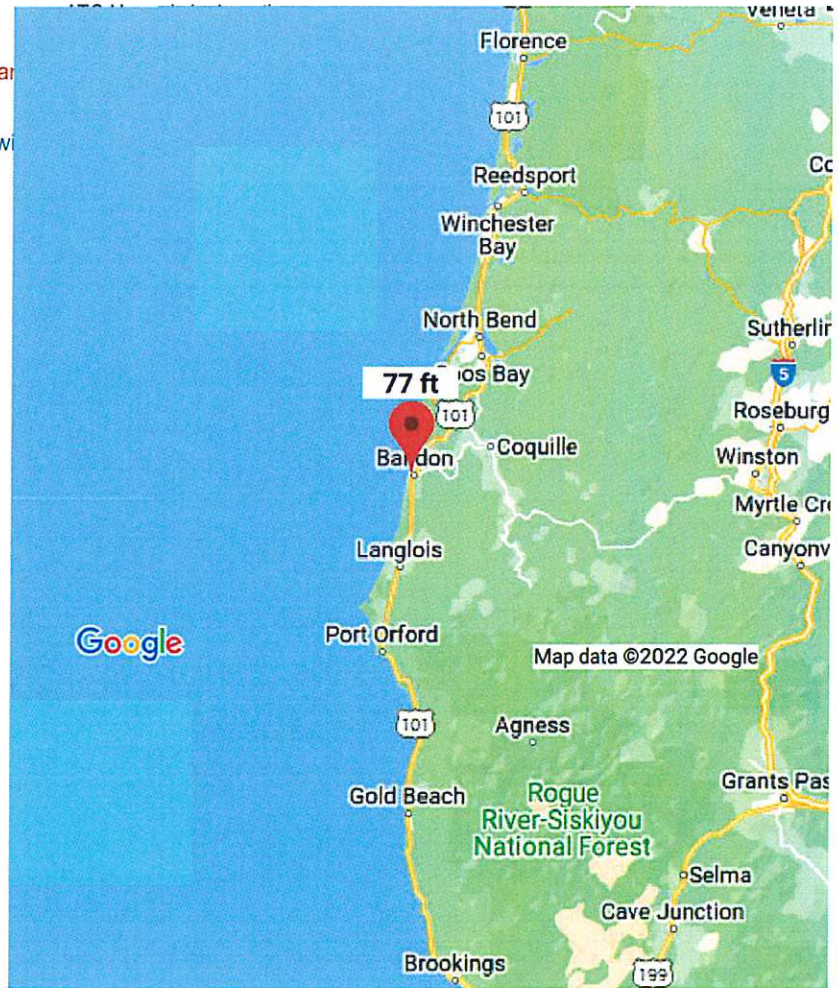
⚠ This is a beta release of the new ATC Hazard

i The ATC Hazards by Location website w

ATC Hazards by Location

Search Information

Address: 455 9th St SW, Bandon, OR 97411, USA
Coordinates: 43.1150088, -124.419577
Elevation: 77 ft
Timestamp: 2022-08-17T18:10:48.678Z
Hazard Type: Seismic
Reference Document: ASCE7-16
Risk Category: II
Site Class: D-default



Basic Parameters

Name	Value	Description
S _S	2.03	MCE _R ground motion (period=0.2s)
S ₁	0.967	MCE _R ground motion (period=1.0s)
S _{MS}	2.436	Site-modified spectral acceleration value
S _{M1}	* null	Site-modified spectral acceleration value
S _{DS}	1.624	Numeric seismic design value at 0.2s SA
S _{D1}	* null	Numeric seismic design value at 1.0s SA

* See Section 11.4.8

Additional Information

Name	Value	Description
SDC	* null	Seismic design category
F _a	1.2	Site amplification factor at 0.2s
F _v	* null	Site amplification factor at 1.0s
CR _S	0.859	Coefficient of risk (0.2s)
CR ₁	0.862	Coefficient of risk (1.0s)
PGA	1.005	MCE _G peak ground acceleration

F_{PGA}	1.2	Site amplification factor at PGA
PGA_M	1.206	Site modified peak ground acceleration
T_L	16	Long-period transition period (s)
SsRT	2.03	Probabilistic risk-targeted ground motion (0.2s)
SsUH	2.364	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
SsD	3.248	Factored deterministic acceleration value (0.2s)
S1RT	0.967	Probabilistic risk-targeted ground motion (1.0s)
S1UH	1.121	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)
S1D	1.258	Factored deterministic acceleration value (1.0s)
PGAd	1.355	Factored deterministic acceleration value (PGA)

* See Section 11.4.8

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Please note that the ATC Hazards by Location website will not be updated to support ASCE 7-22. [Find out why.](#)

Disclaimer

Hazard loads are provided by the U.S. Geological Survey [Seismic Design Web Services](#).

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GENERAL BUILDING DATA:

ITEMS	FEET
WIDTH:	42'-0"
LENGTH:	50'-0"
EAVE HEIGHT:	16'-3"
ROOF PITCH:	4"
FRAME SPACING:	10'-0"
RAFTER SPACING:	2'-0"
GIRT SPACING:	2'-0"

GENERAL NOTES

THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE OWNER /CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DESCREPENCIES. CHANGES, OMISSIONS OR SUBSTITUTIONS ARE NOT PERMITTED WITHOUT THE APPROVAL OF THE BUILDING DEPARTMENT HAVING JURISDICTION. ALL WORKMANSHIP SHALL CONFORM TO OREGON STRUCTURAL SPECIALTY CODE, 2019 EDITION. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION . THE CONTRACTOR/OWNER IS RESPONSIBLE FOR MEHODS AND/ SEQUENCES OF ASSEMBLING THE STRUCTURE. THE CONTRACTOR/OWNER IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE DURING THE CONSTRUCTION AND PRIOR TO COMPLETION OF ALL SHEAR WALLS, ROOF AND FLOOR DIAPHRAGMS

DRILLED FOUNDATION

THE FOOTING SHALL BEAR ON NATIVE, INORGANIC, UNDISTURBED NATIVE SOIL BELOW EXISTING GRADE. ALL STRUCTURAL FOOTINGS SHALL BE EXTENDED MINIMUM 4'-0" BELOW FINISHED GRADE UNLESS NOTED OTHERWISE ON PLANS. THERE SHALL BE 95% COMPATION OF ALL BACKFILL FOR SLAB ON GRADE IN ACCORDANCE OF ASTM D1557 MODIFIED PROCTOR DENSITY.

BACK FILL IN THE ANNULAR SPACE AROUND THE POSTS SHALL BE BY ONE OF THE FOLLOWING METHODS:

- A. CONCRETE WITH THE ULTIMATE STRENGTH OF MIN. 2000 PSI.
- B. CLEAN SAND: THE SAND SHALL BE COMPACTED BY TAMPING IN LAYERS NOT MORE THAN 8" IN DEPTH.
- C. GRANULAR BACKFILL: THE GRANULAR BACKFILL SHALL BE 3/4 (-) GRAVEL OR CRASHED ROCK.

DESIGN CRITERIA

ITEMS	UNIT
WIND SPEED	120 MPH
EXPOSURE	B
SEISMIC_ZONE	D
DEAD LOAD	5 psf
SNOW LOAD	25 psf
SOIL BEARING	1500 psf
BUILDING_CODE	IBC 2018

BACKFILL SHALL BE COMPACTED BY TAMPING IN LAYERS NOT MORE THAN 8" IN DEPTH.

D. NO SPECIAL INSPECTION FOR COMPATION BY TAMPING

WOOD

STRUCTURAL LUMBER SHALL CONFORM TO WESTERN SOFTWOOD ASSOCIATION FOR GRADING.
 -POSTS SHALL BE 6 X 6 P.T HF#2 U.N.O
 -POSTS SHALL BE PRESSURE TRATED TO 0.60 pcf
 RETENTION CCA ACCORDANCE WITH UBC STD 25-12 AND A.W.P.A LP-44

-POSTS SHALL BE CENTERED ON THE FOOTING.
 -GIRTS SHALL BE 2 X 6 DF#2 @ 24" O.C U.N.O
 -RAFTERS SHALL BE 2 X 6 DF#2 @ 24" O.C U.N.O

FASTENERS, ANCHORS AND CONNECTORS

BOLTS SHALL BE 3/4 DIAMETER MACHINE BOLTS WITH WASHERS AND NUTS AND SHALL CONFORM TO ASTM A-30-D SPCS. WHERE BOLTS COME INTO CONTACT WITH PRESSURE TREATED WOOD WHEN CHEMONITE IS USED., THEY SHALL BE GALVANIZED STEEL. NUTS FOR BOLTS SHALL BE EITHER CROWN NUTS OR NYLON LOCKNUTS AND SHALL BE TIGHTENED TO THE APPROPRIATE MANUFACTURER'S RECOMMENDATIONS.

NAILS IN TREATED WOOD SHALL BE HOT DIPPED GALVANIZED.

GIRT TO POST RAFTER TO BLOCKING BLOCKING TO TRUSS (NAILS AT EACH SIDE U.N.O)	(3)16d
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CONCRETE SLAB ON GRADE IS NOT DESIGNED BY S&T CONSULTING.

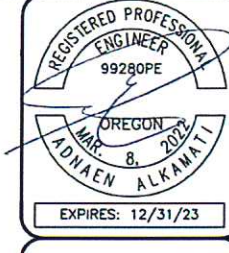
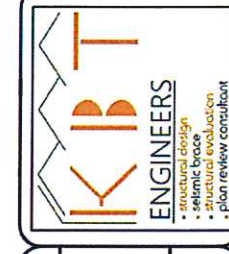
IT IS THE RESPONSIBILITY OF THE CONTRACTOR OR THE OWNER. THE SUBGRADE FOR THE SLAB SHALL BE PREPARED ACCORDING TO ASTM D1557. CONTROL JOINTS ARE RECOMMENDED TO CONTROL CRACKING.

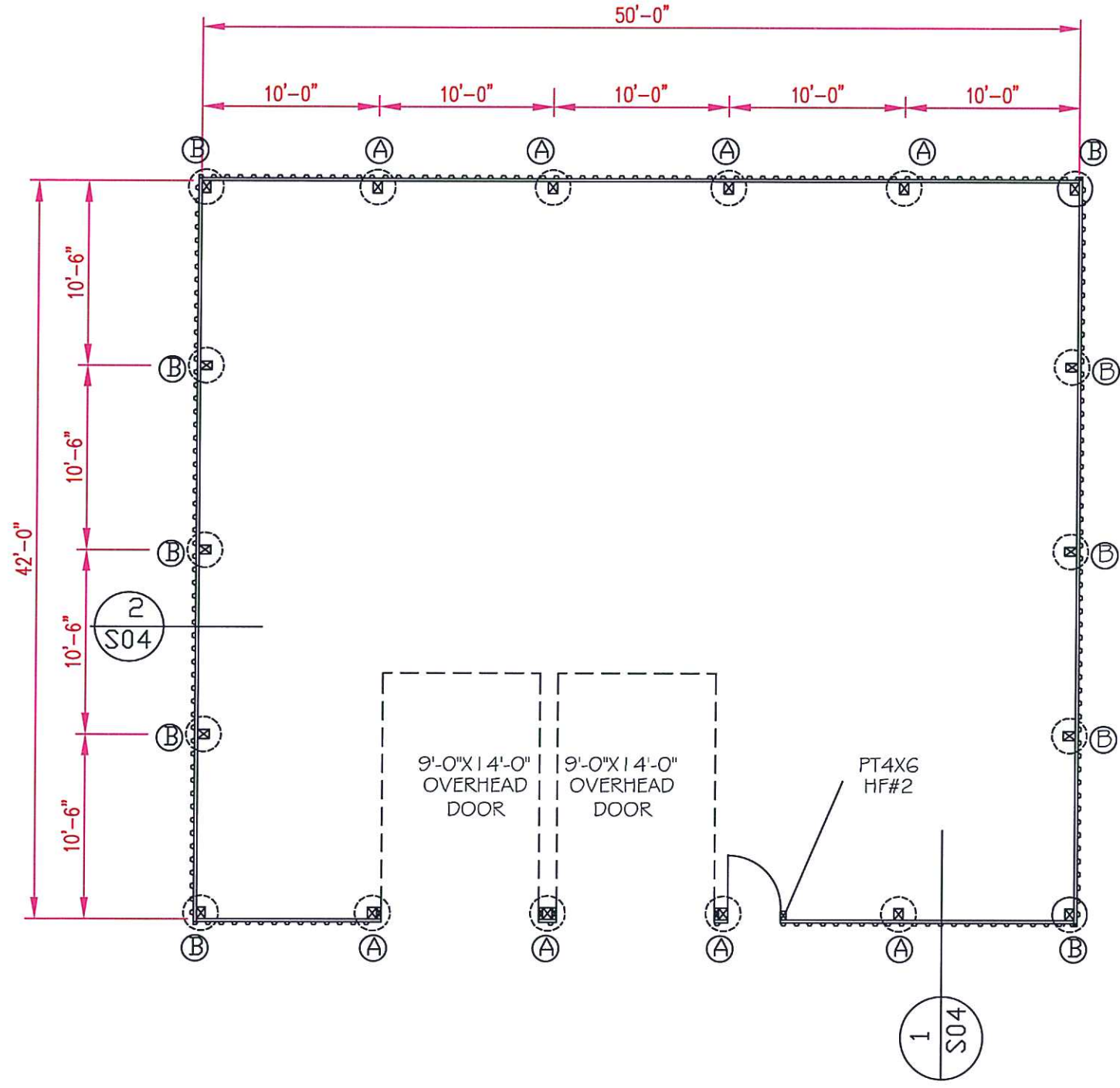
SHEET No
501

JOB No
BJS-2021-05

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	PLOTTING SCALE:

42'-0" X 50'-0" X 16'-3"
 BANDON SCHOOL DISTRICT
 455 9TH STREET SW BANDON OR 97411
 BJS METAL & LUMBER PRODUCTS, LLC





NOTE:
 THIS BUILDING WAS PERMITTED AND BUILT AS ROOF ONLY WITH ALL FOUR SIDES OPEN.
 THIS SCOPE OF THIS PERMIT/WORK IS TO ENCLOSE THE BUILDING AND PROVIDE 2 OVERHEAD DOORS AND ONE MAN DOOR

POST	POST SIZE	HOLE DIA (IN)	HOLE DEPTH (IN)	REMARKS
(A)	EXISTING POSTS P.T 6X6 HF#2	29	54	SEE DETAIL 2/S07
(B)	EXISTING POSTS P.T 6X8 HF#2	21	54	SEE DETAIL 2/S07

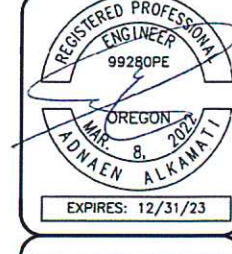
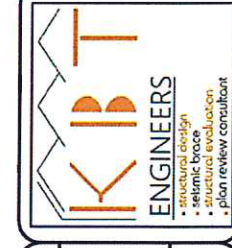
1 FOUNDATION/FLOOR PLAN

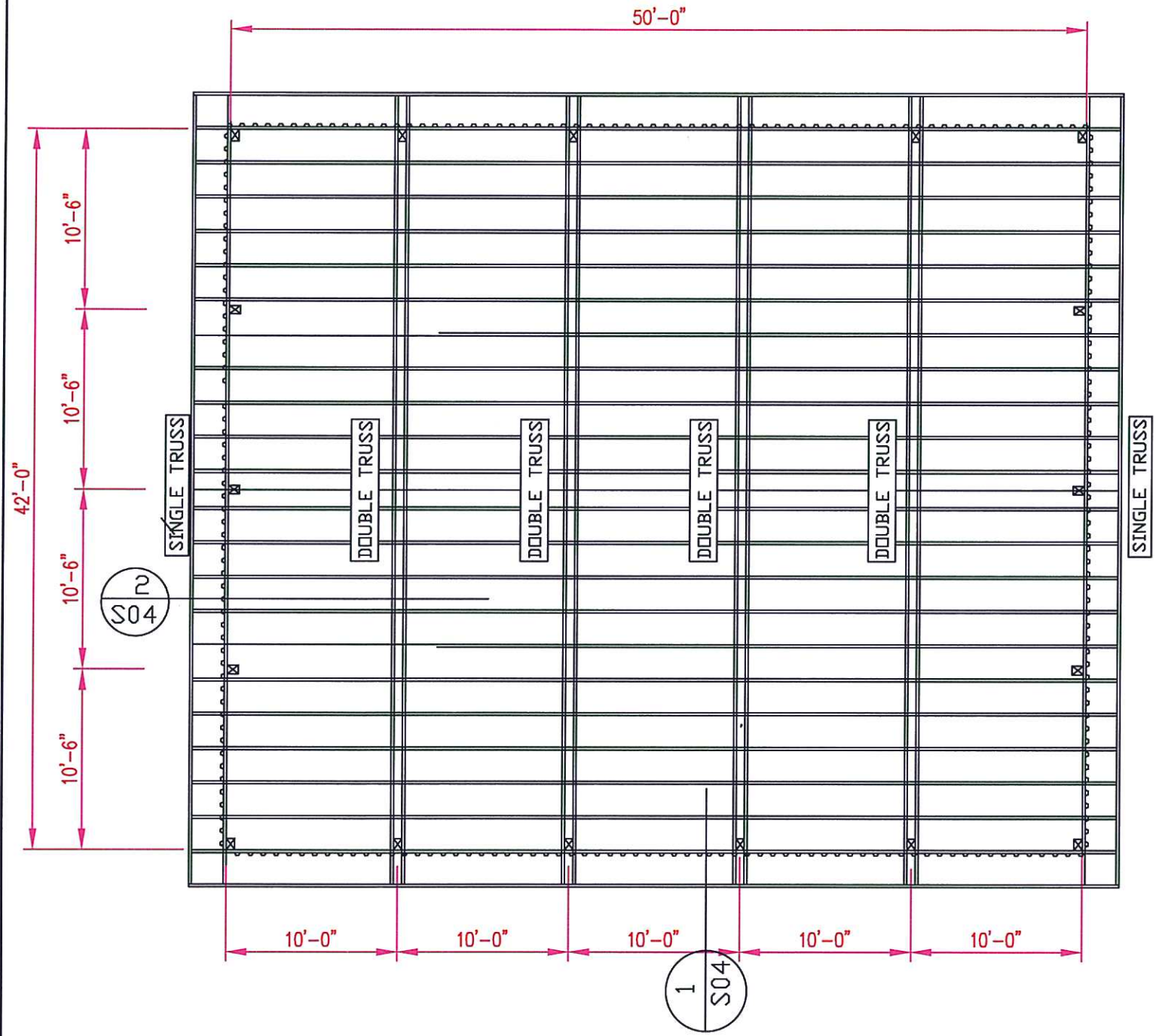
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JOB No:
BJS-2021-05

SHEET No:
S02

42'-0" X 50'-0" X 16'-3"
 BANDON SCHOOL DISTRICT
 455 9TH STREET SW BANDON OR 97411
 BJS METAL & LUMBER PRODUCTS, LLC





NOTE:
 THIS BUILDING WAS PERMITTED AND BUILT AS ROOF ONLY WITH ALL FOUR SIDES OPEN.
 THIS SCOPE OF THIS PERMIT/WORK IS TO ENCLOSE THE BUILDING AND PROVIDE 2 OVERHEAD DOORS AND ONE MAN DOOR

PLAN NOTES:

1. EXISTING ROOF RAFTERS: 2X6 DF #2 @ 24" O.C
2. EXISTING 29 GAGE ROOF SHEATHING ATTACHED TO FRAMING PER S08.
3. 7/16 OSB WITH 8d@6" O.C EDGES AND 12" O.C FIELD (OPTIONAL)
4. 15# FELT
5. 2'-0" MAX OVERHANG (OPTIONAL)

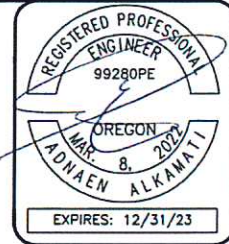
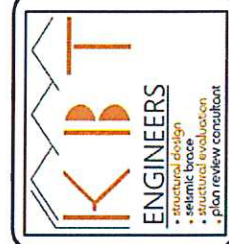
1 ROOF FRAMING PLAN
 S03

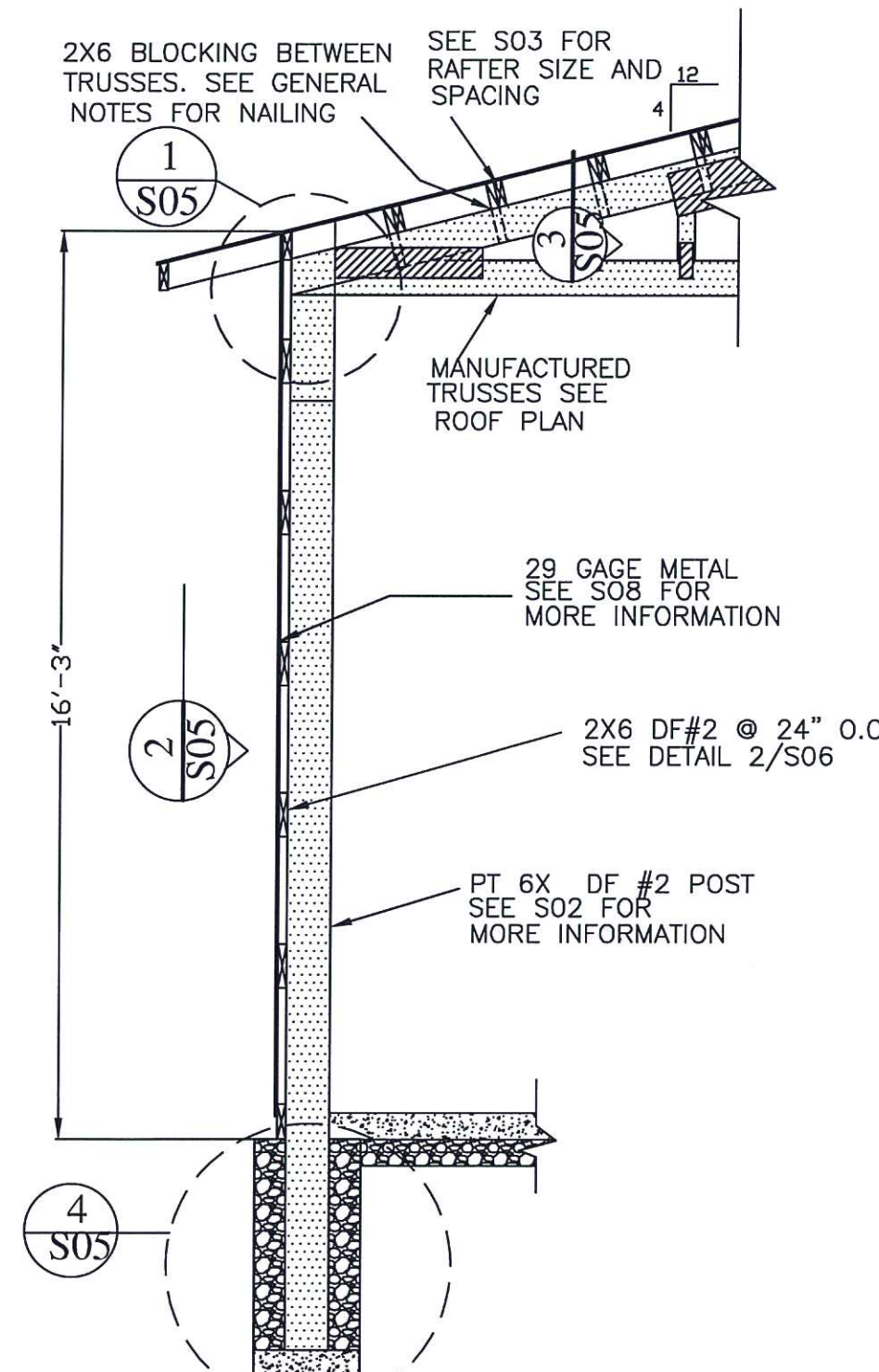
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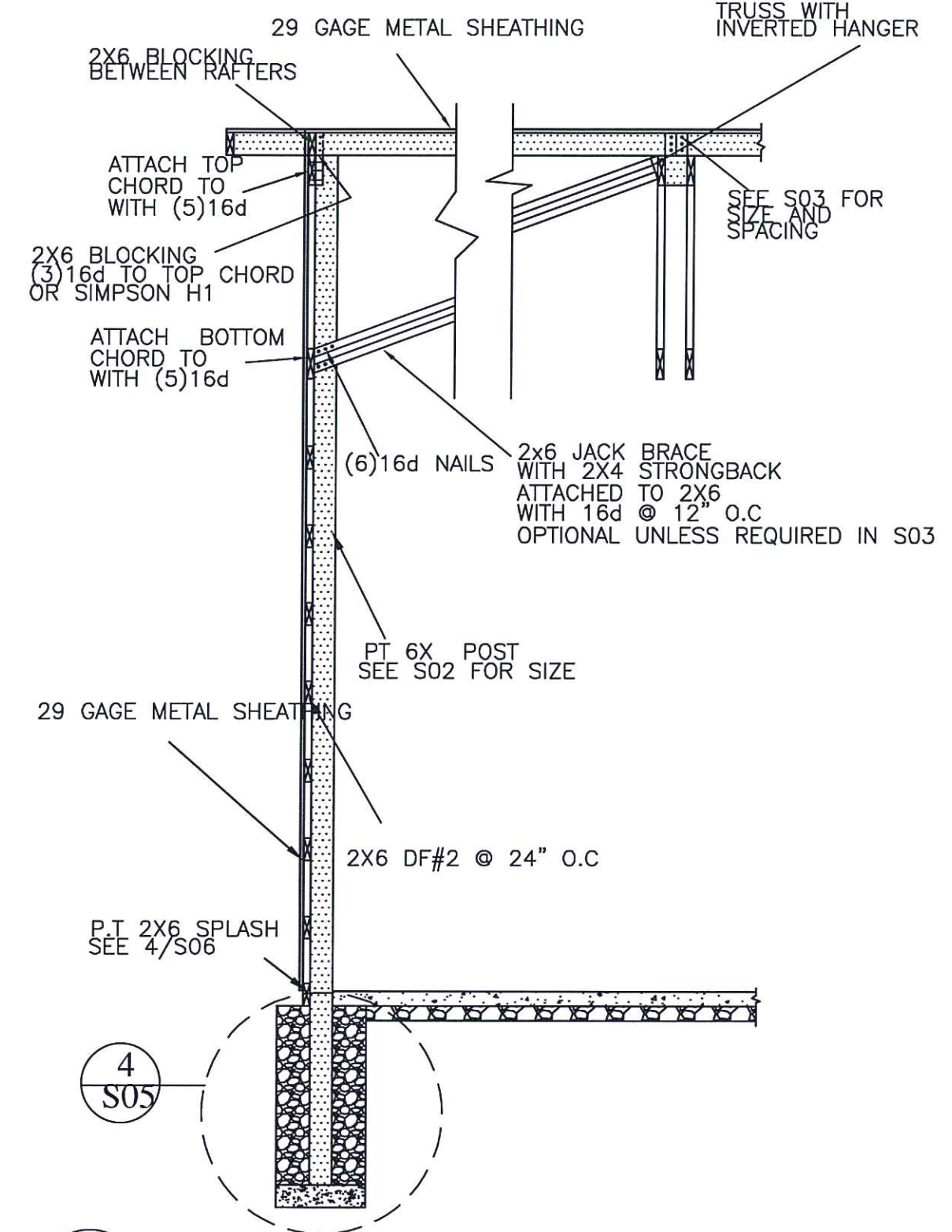
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42'-0" X 50'-0" X 16'-3"
 BANDON SCHOOL DISTRICT
 455 9TH STREET SW BANDON OR 97411
 BJS METAL & LUMBER PRODUCTS, LLC

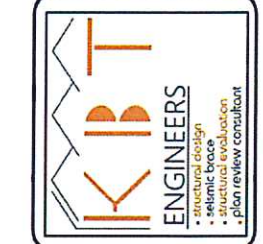




1
S04
TYPICAL FRAME SECTION/ SIDE WALL



2
S04
TYPICAL FRAME SECTION/ GABLE WALL

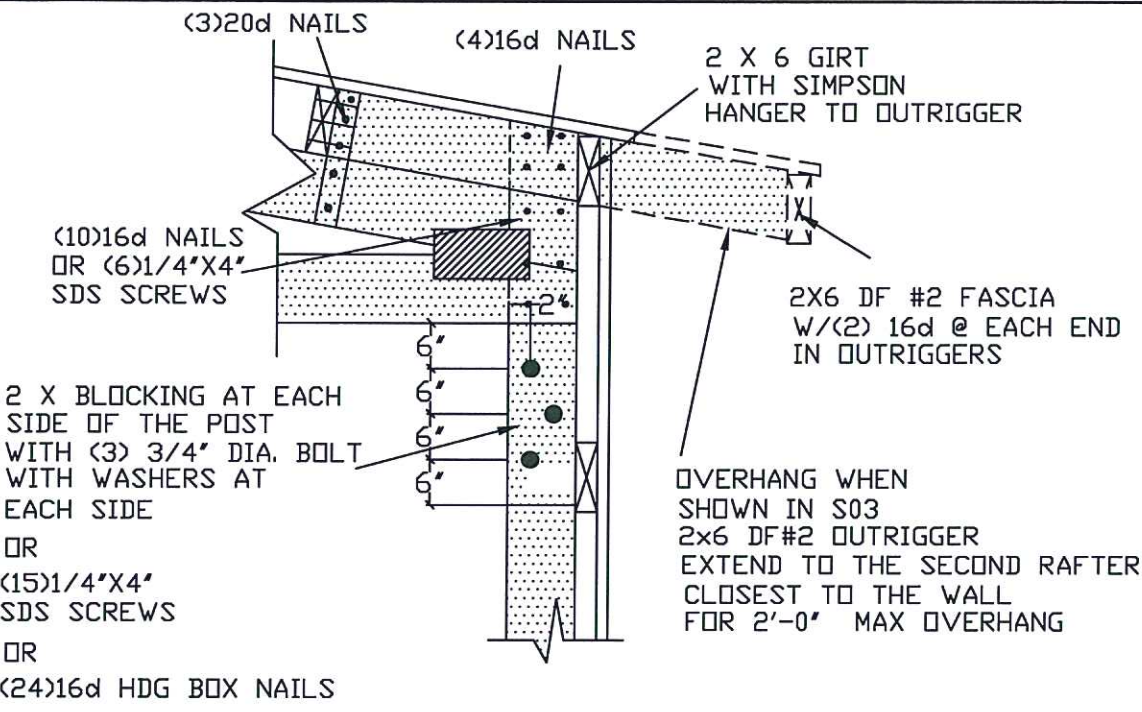


42'-0" X 50'-0" X 16'-3"
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455 9TH STREET SW BANDON OR 97411
BJS METAL & LUMBER PRODUCTS, LLC

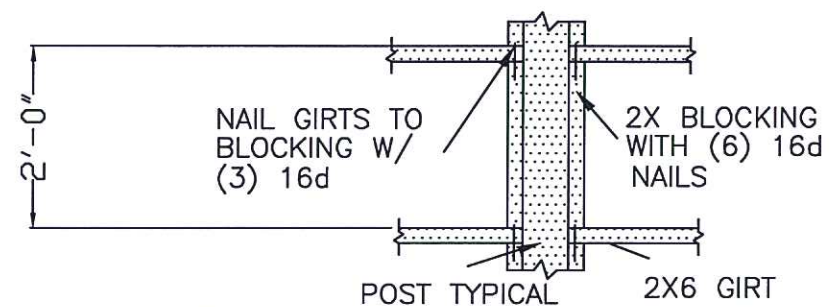
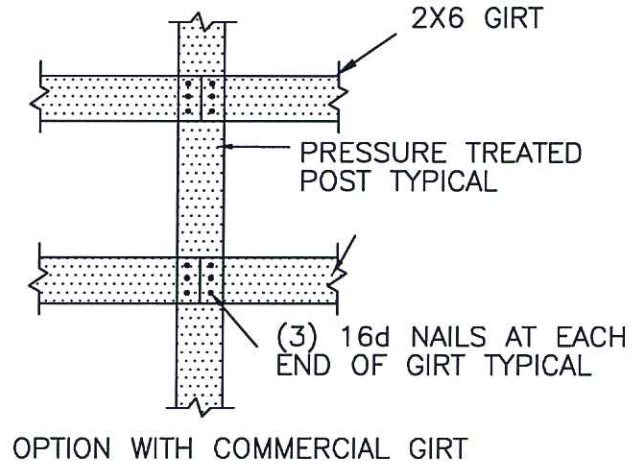
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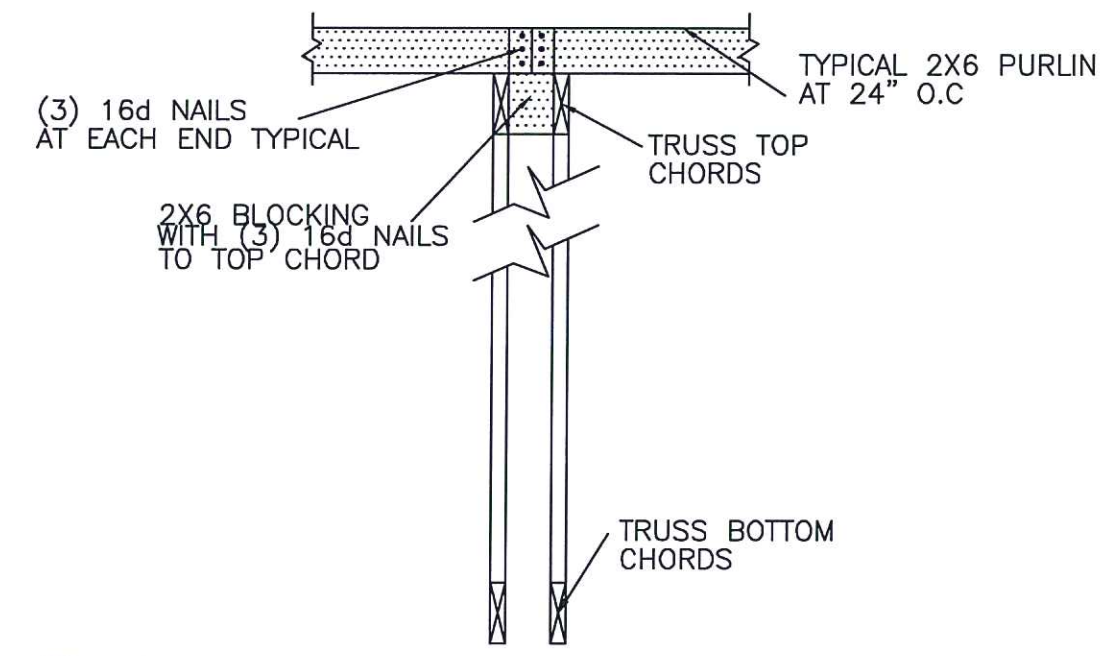
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S04



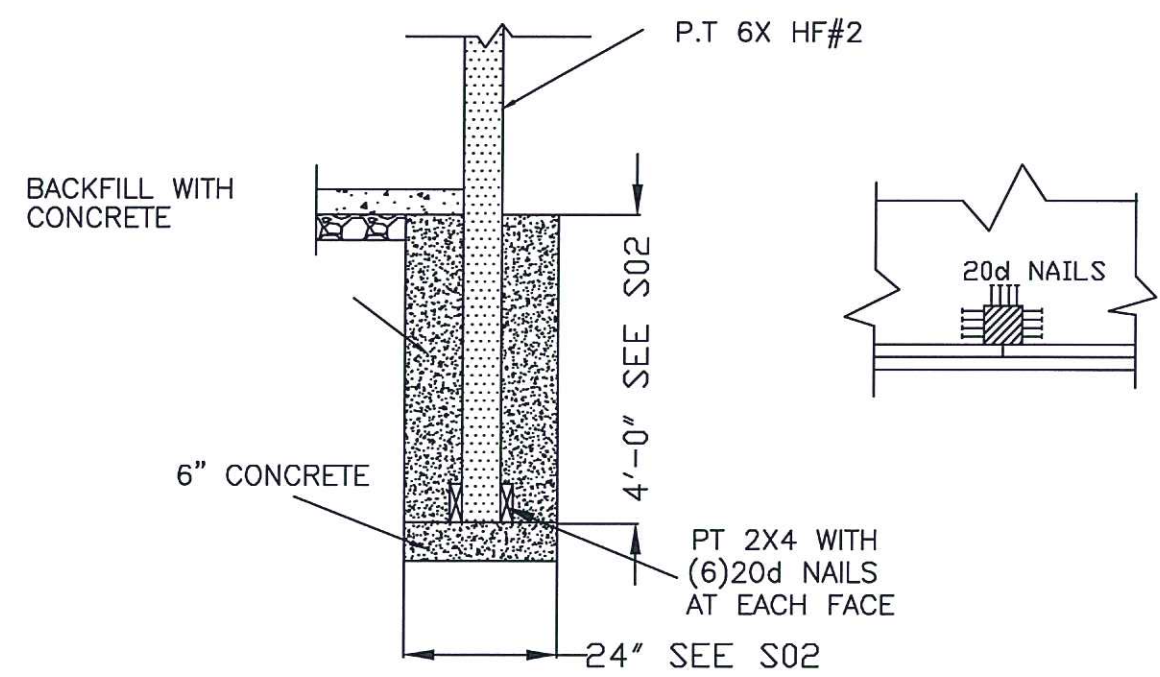
1 TRUSS TO POST CONNECTION DETAIL
S05



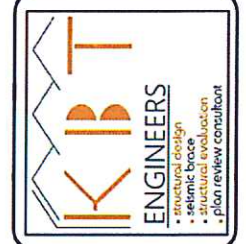
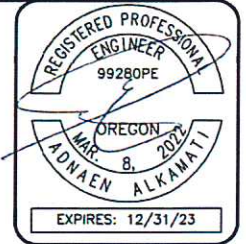
2 GIRT TO POST CONNECTION DETAIL
S05



3 TYPICAL PURLIN TO TRUSS CONNECTION DETAIL
S05



4 FOUNDATION DETAIL
S05

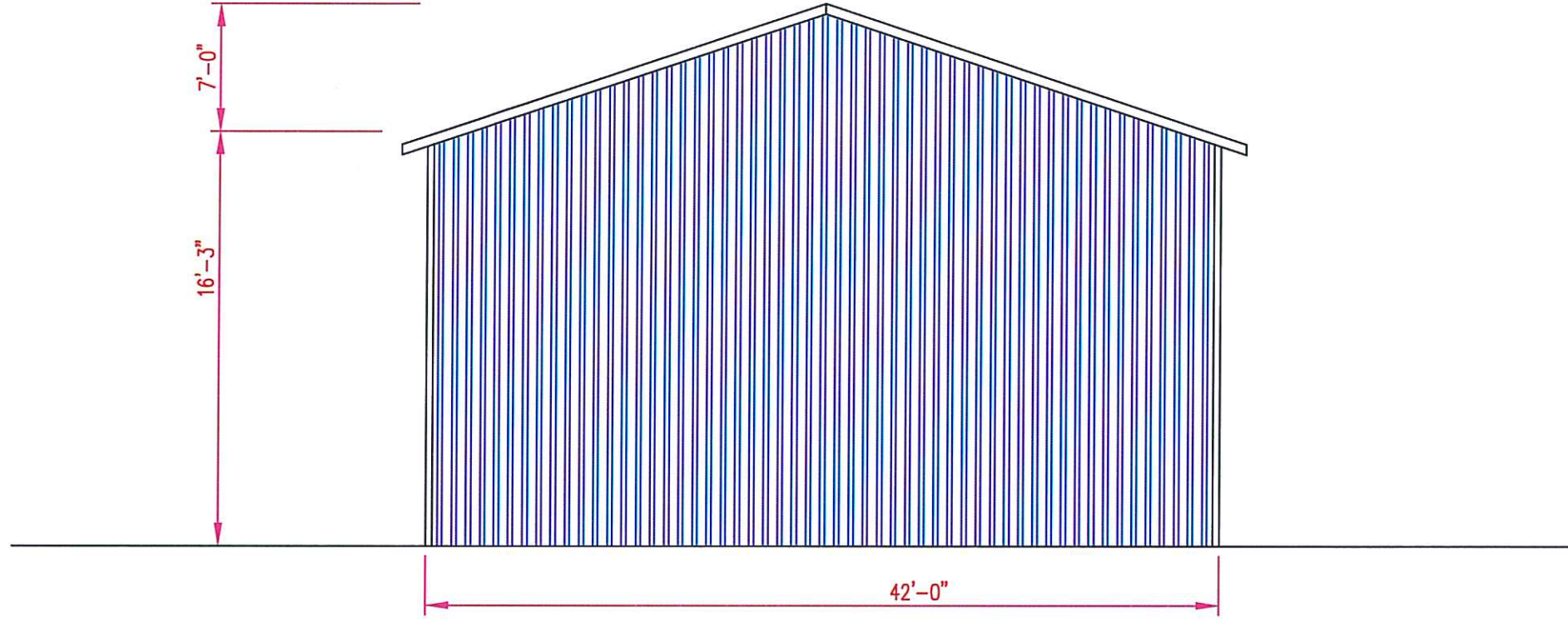


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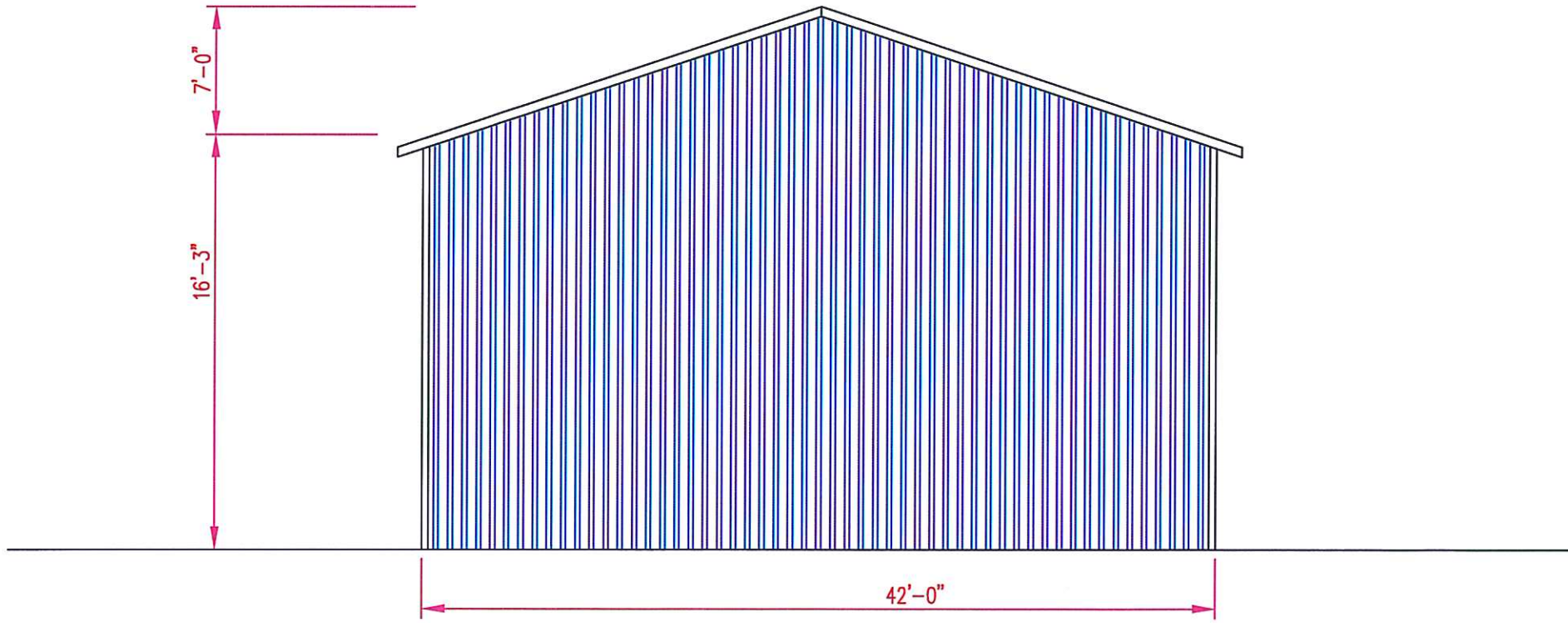
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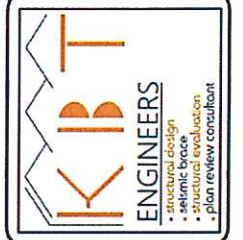
SHEET No:
S05



1 LEFT ELEVATION
S08



2 RIGHT ELEVATION
S08



42'-0" X 50'-0" X 16'-3"
BANDON SCHOOL DISTRICT
455 9TH STREET SW BANDON OR 97411
BJS METAL & LUMBER PRODUCTS, LLC

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SHEET No:
S08

NOTES

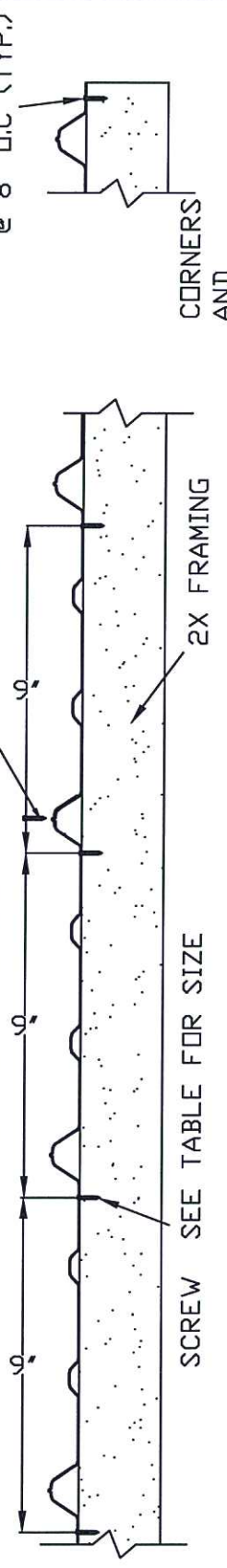
1. ALL ROOF AND SIDE, AND GABLE WALL METAL SHEATHING SHALL BE MINIMUM 29 GAGE WITH RIP PATTERN AT 9" INTERVALS FASTEN METAL SHEATHING TO EACH GIRT AND PURLIN AS SHOWN BELOW OR ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS WHICHEVER IS MORE RESTRICTIVE.
2. BLOCKING IS REQUIRED BETWEEN GIRTS AND PURLINS IN ORDER TO COMPLY WITH SCREW SPACING AT CORNERS AND OPENINGS,
3. STICH SCREWS MAY BE USED ON SEAM RIB IF DESIRED OR OTHERWISE REQUIRED BY DESIGN (SEE PLANS), BUT NOT NECESSARY IF WALL AND ROOF PANEL LAPS ARE PLACED IN THE DIRECTION OF PREVAILING WIND.

PREVAILING WIND DIRECTION →

← APPLY IN THIS DIRECTION

#9X 3/4" STITCH SCREWS @ 8" O.C
WHEN REQUIRED SEE NOTE ON ELEVATIONS

#9 X 1" SCREWS @ 8" O.C (TYP.)



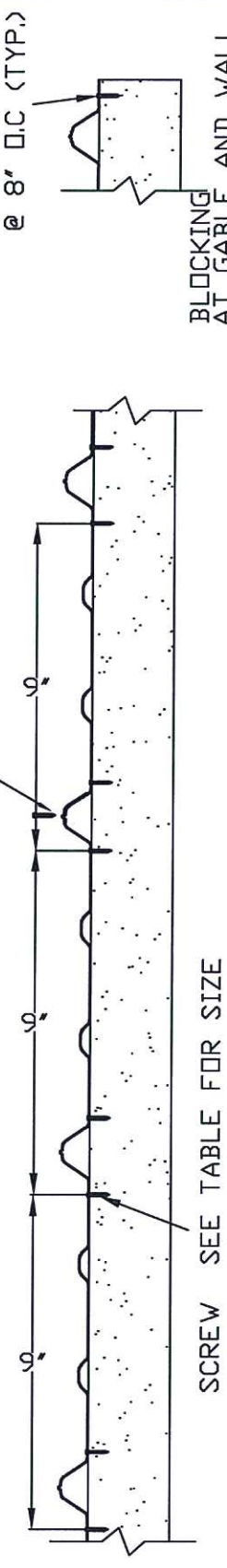
SCREW SEE TABLE FOR SIZE

CORNERS AND OPENINGS

ROOF INTERMEDIATE PURLINS	# 9 X 1" @ 9" O.C
SIDE WALL INTERMEDIATE GIRTS	# 9 X 1" @ 9" O.C
GABLE WALL INTERMEDIATE GIRTS	# 9X 1" @ 9" O.C

#9X 3/4" STITCH SCREWS @ 8" O.C
WHEN REQUIRED SEE NOTE ON ELEVATIONS

#9 X 1" SCREWS @ 8" O.C (TYP.)

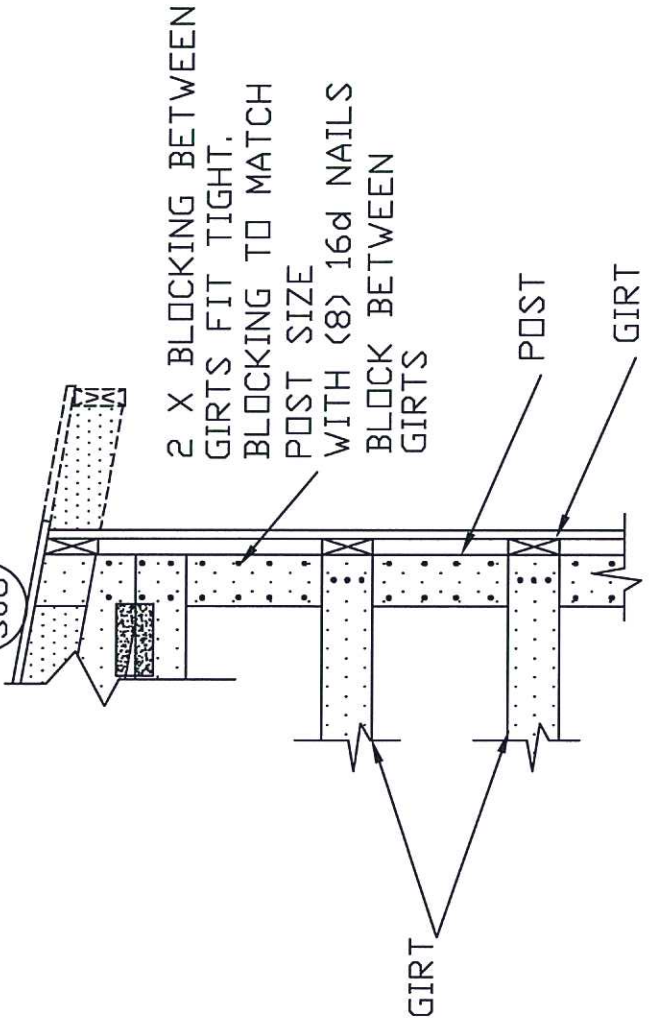


SCREW SEE TABLE FOR SIZE

BLOCKING AT GABLE AND WALL AND OPENINGS

SIDE WALL EAVE AND SPLASH BOARD	# 9 X 1-1/2"
GABLE WALL TRUSS TOP CHORD AND SPLASH BOARD	# 9X 1-1/2"
RIDGE	# 9 X 1-1/2"

1 S06 WALL AND ROOF SHEATHING FASTENING DETAIL



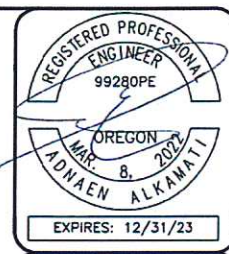
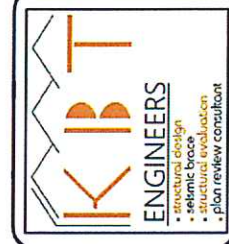
2 S06 GABLE END WALL TRUSS CONNECTION

SHEET No:
S06

JOB No:

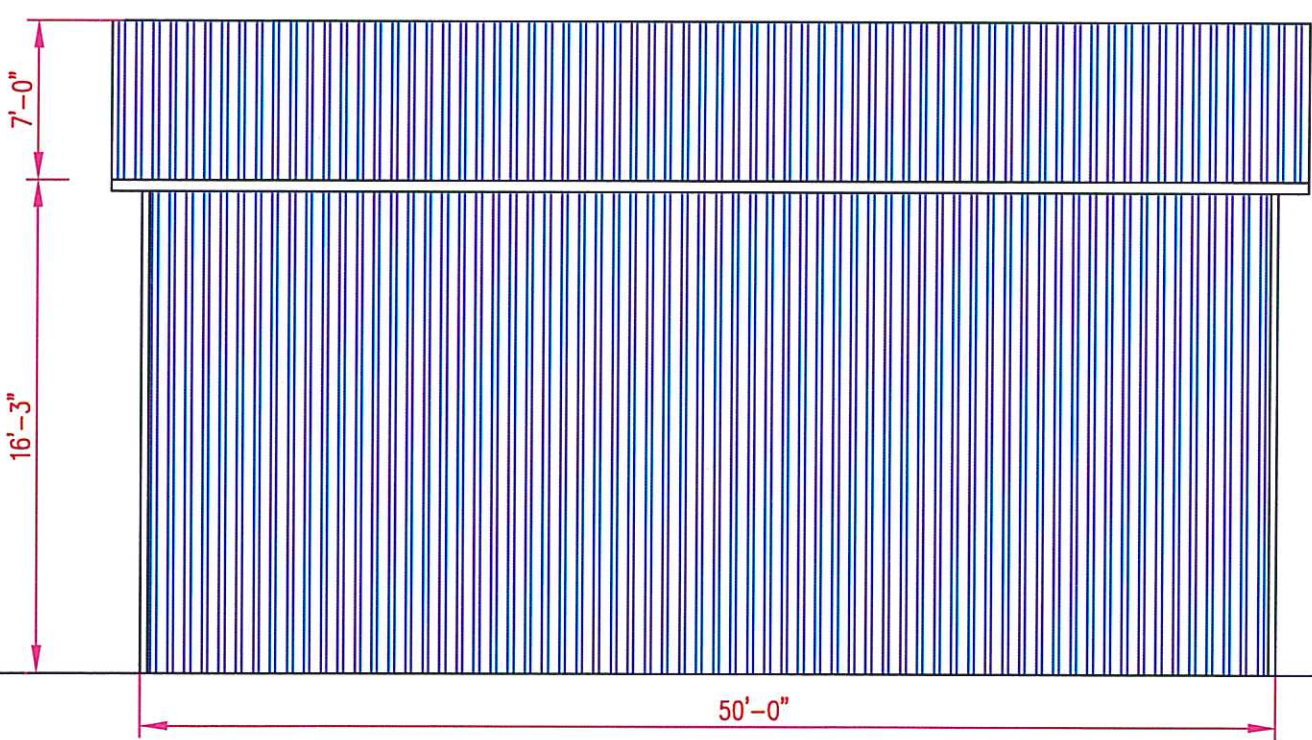
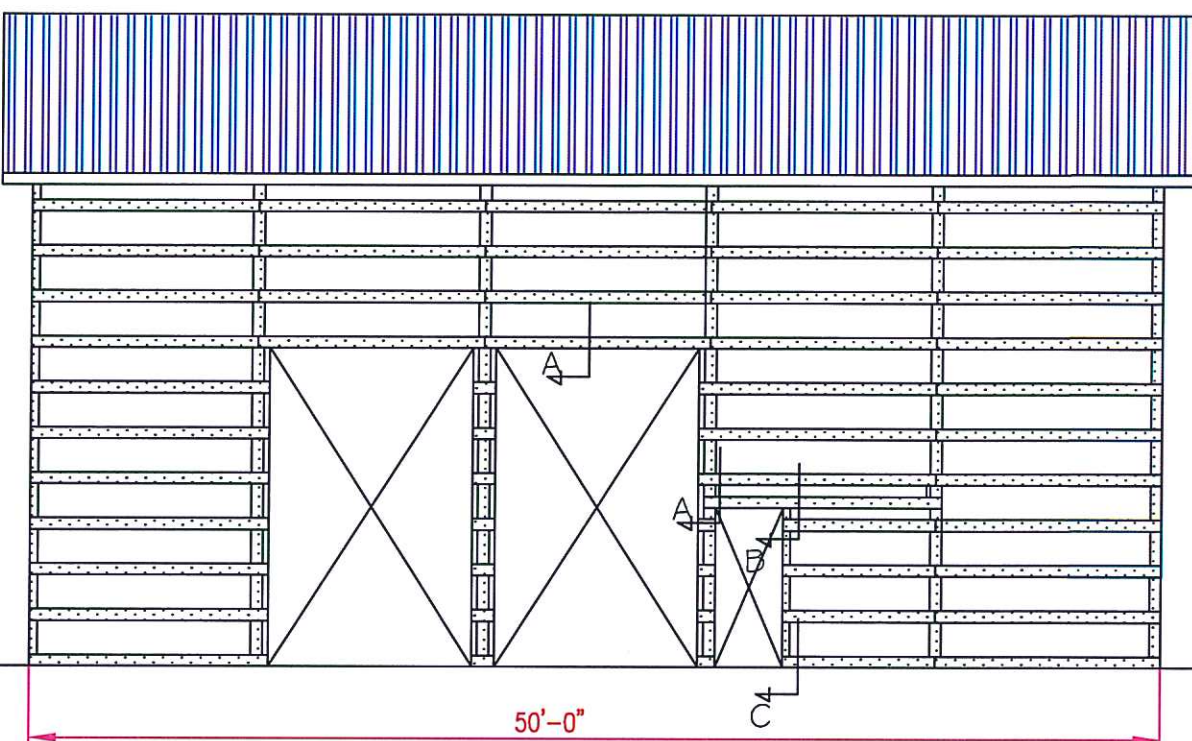
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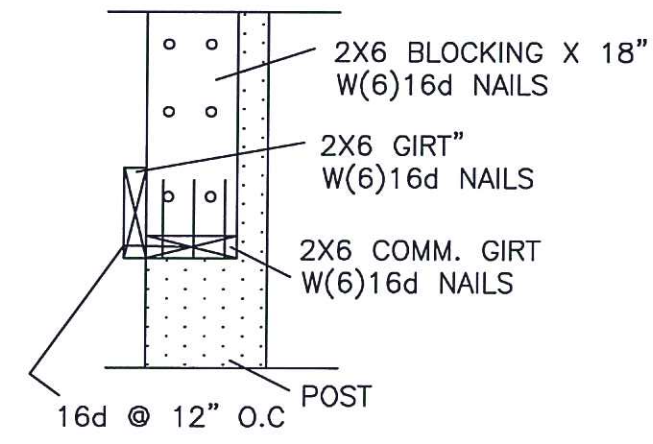




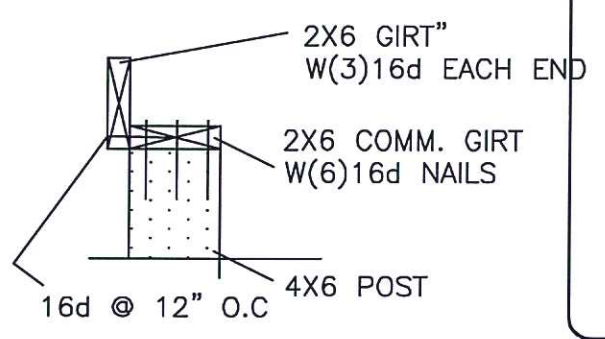
1 FRONT ELEVATION
S07



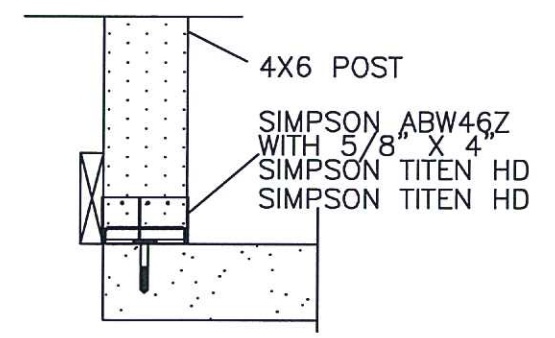
2 BACK ELEVATION
S07



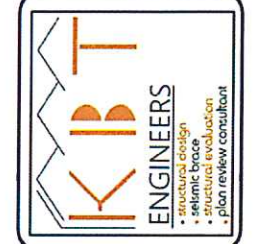
SECTION "A"



SECTION "B"



SECTION "C"



42'-0" X 50'-0" X 16'-3"
BANDON SCHOOL DISTRICT
455 9TH STREET SW BANDON OR 97411
BJS METAL & LUMBER PRODUCTS, LLC

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SHEET No:
S07